# **Work Plan**

# Borehole Backfilling Within Chicopee Falls Levee System Right-of-Way

Former Facemate Corporation Facility 5 West Main Street Chicopee, Massachusetts 01020

November 16, 2010

Prepared for Submittal to:

United States Army Corps of Engineers 696 Virginia Road Concord, MA 01742-2751

Prepared on Behalf of:

City of Chicopee Office of Community Development 38 Center Street Chicopee, Massachusetts 01020

Prepared by:



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- 2a Site Plan: U.S. Army Engineer Division Drawing Number CT-5885 Sheet 2
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## **1.0 Introduction**

On behalf of the Chicopee Office of Community Development (OCD), BETA Group, Inc. (BETA) has prepared this Work Plan for the backfilling of up to two boreholes recently installed within or near the right-of-way of the Chicopee Falls Levee System at the former Facemate Facility property located at 5 West Main Street in Chicopee, Massachusetts. It is also intended that this Work Plan will define a protocol for communicating and coordinating prior approval of future activities within that right-of-way.

BETA is under contract with the City of Chicopee to provide Licensed Site Professional (LSP) and related services in to identify and assess environmental conditions at the Former Facemate Facility and Uniroyal properties (the "Site"). BETA will also be providing LSP services related to remediation (and clean-up) activities.

BETA understands that the United States Army Corps of Engineers (USACE) maintains jurisdiction over, and oversees operation and maintenance of, the Chicopee Falls Levee System. The Chicopee Falls project provides flood protection for the area of Chicopee along the right (north) bank of the Chicopee River about two miles upstream from the mouth of the river (Chicopee Falls). The project consists of about 4,800 feet of levees and walls, two pressure drains for discharging high level interior drainage, and two pumping stations for storm water. The Chicopee Falls project was built following the Chicopee River flood of August 1955. Construction was started in October 1963 and completed in July 1965.

USACE has jurisdiction over the levee system and approval from USACE is required prior to conducting certain activities within the levee right-of-way.

Numerous sources of both oil and hazardous material have been present at the Site throughout its industrial history, including large capacity underground storage tanks containing #6 fuel oil, above-ground storage tanks containing hydrogen peroxide and muriatic acid, and both non-PCB and PCB-containing transformers. Soil contamination identified to date includes PCBs, heavy metals, Polynuclear Aromatic Hydrocarbons (PAHs), Volatile Organic Compounds (VOCs), and petroleum hydrocarbons.

BETA is currently undertaking supplemental site investigations to help assess the nature and extent of environmental contamination at the Site that may represent significant risk to human health, safety and/or the environment. Those supplemental investigation activities include, among other things, subsurface explorations (drilling of soil borings, installation of monitoring wells, excavation of test pits, etc.), some of which will need to be within or in close proximity to the right-of-way of the Chicopee Falls Levee System. As of the date of this Work Plan, BETA subsurface investigation has occurred at only one location (SB-11) that is within 50 feet of the toe of the levee.

The full extent and location of remediation activities is not currently known, but will be dependent on the findings of the ongoing investigations. BETA is currently in the design phase of a project to remove underground storage tanks (USTs) located in the inner (northwest) corner of the "elbow" formed by Buildings 1 and 5 as shown on the USACE Drawing Number CT-5885 Sheet 2 (June 1963 Corps Chicopee Falls Flood Control Plans). There are three USTs to be removed, none of which is proximate to the levee; the nearest tank to the levee is approximately 70 feet to the south of the 24-inch R.C.P. interceptor drain and approximately 90 feet to the south of the toe of the levee. Despite the buffer distance to the levee, the Contract Documents for the UST removal project will include notification to the contractor of the levee system, define a limit of work that provides a separation from the levee system (including the interceptor drain), and stipulate that the contractor's activities are strictly prohibited beyond that limit of work.

## 2.0 Summary of Work Completed to Date

BETA was on Site on August 26, 2010 to complete soil borings as part of the ongoing supplemental investigation. Two of these borings, SB-10 and SB-11, were completed at the western side of the property; SB-10 is approximately 50 feet east of the 18-inch R.C.P. interceptor drain ( $57\pm$  feet east of the toe of the levee), and SB-11 is approximately five (5) feet east of the 18-inch R.C.P. interceptor drain ( $18\pm$  feet east of the toe of the levee). A Site Locus is included as Figure 1 and Site Plans<sup>1</sup> are included as Figures 2a and 2b (see figure 2a for boring locations). The borings were drilled using 4-3/8 inch (ID) hollow stem augers with a 140-pound hammer to drive 1-3/8 inch split barrel sampler. Boring SB-10 was advanced to fourteen (14) feet below ground surface (bgs) and boring SB-11 was advanced to eighteen (18) feet bgs. Both borings were backfilled upon completion; however, the method of backfilling did not meet USACE requirements. Soil boring logs are included in Appendix A.

### **3.0** Corrective Actions at SB-11

BETA proposes to re-drill soil boring SB-11 and backfill it in accordance with USACE Regulation ER 1110-1-1807 (Procedures for Drilling in Earth Embankments). The borehole will be re-drilled using 10-inch I.D. (inside diameter) driven casing (drive-and-wash methods) to 18 feet, the depth of the original boring. Since the 10-inch I.D. driven casings are larger than the 8-inch O.D. (outside diameter) "flight" diameter of the hollow-stem augers that were used to drill the original boring, any disturbed/loosened soil along the sidewall of the original boring will be captured and removed. Grout conforming to the requirements of USACE Engineering Manual EM 1110-2-3506 (Grouting Technology) will be injected through a tremie pipe or hose inserted

<sup>&</sup>lt;sup>1</sup> The Site Plans for this Work Plan are based on the drawings entitled "Connecticut River Flood Control Project, Chicopee Falls, Mass., Plans for the Local Protection Project", dated June 1963 (U.S. Army Engineer Division, New England, Corps of Engineers, Waltham, Mass.). Figure 2a is a reduction of Drawing Number CT-5885 Sheet 2, scaled to fit an 11 inch by 17 inch sheet. Figure 2b is a blow-up of a portion of Figure 2b; the area of coverage of Figure 2b is shown on Figure 2a.

to a depth of six (6) inches above the base of the boring. The hole will then be grouted to twelve (12) inches below the ground surface. The quantity of backfill grout will be estimated based on the depth and diameter of the bore hole prior to starting backfilling operations, and injection quantities will be monitored continuously. If the estimated quantity per linear foot of hole is exceeded significantly, operations will be halted, the casing will be pulled to the top of the grout, and the grout will be allowed to set; once the grout has set, the grouting operations will resume.

### 4.0 As-Built Plan

The re-drilling and grouting of each borehole will be logged; the log and an as-built plan showing the location of the grouted hole will be prepared and submitted to USACE following completion.

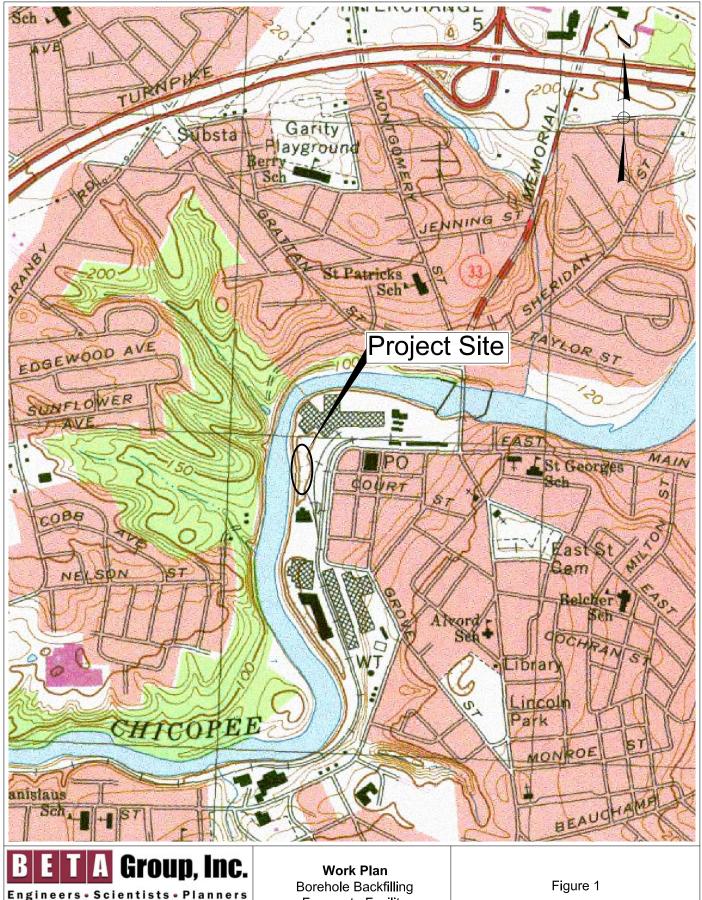
## 5.0 Future Subsurface Activities

As indicated above, supplemental subsurface investigation (drilling of soil borings, installation of monitoring wells, excavation of test pits, etc.) and remediation activities will be ongoing at the Site; some of those activities will need to be within or in close proximity to the right-of-way of the Chicopee Falls Levee System.

Prior to commencing any future activities within the right-of-way, an activity-specific work plan will be prepared and submitted for review and approval by USACE. Such work plans will be submitted at least two weeks prior to the date the activities are scheduled to start, and will be submitted to USACE, with a copy to Thomas M. Hamel, Chief Operator of the Chicopee Water Pollution Control Department.

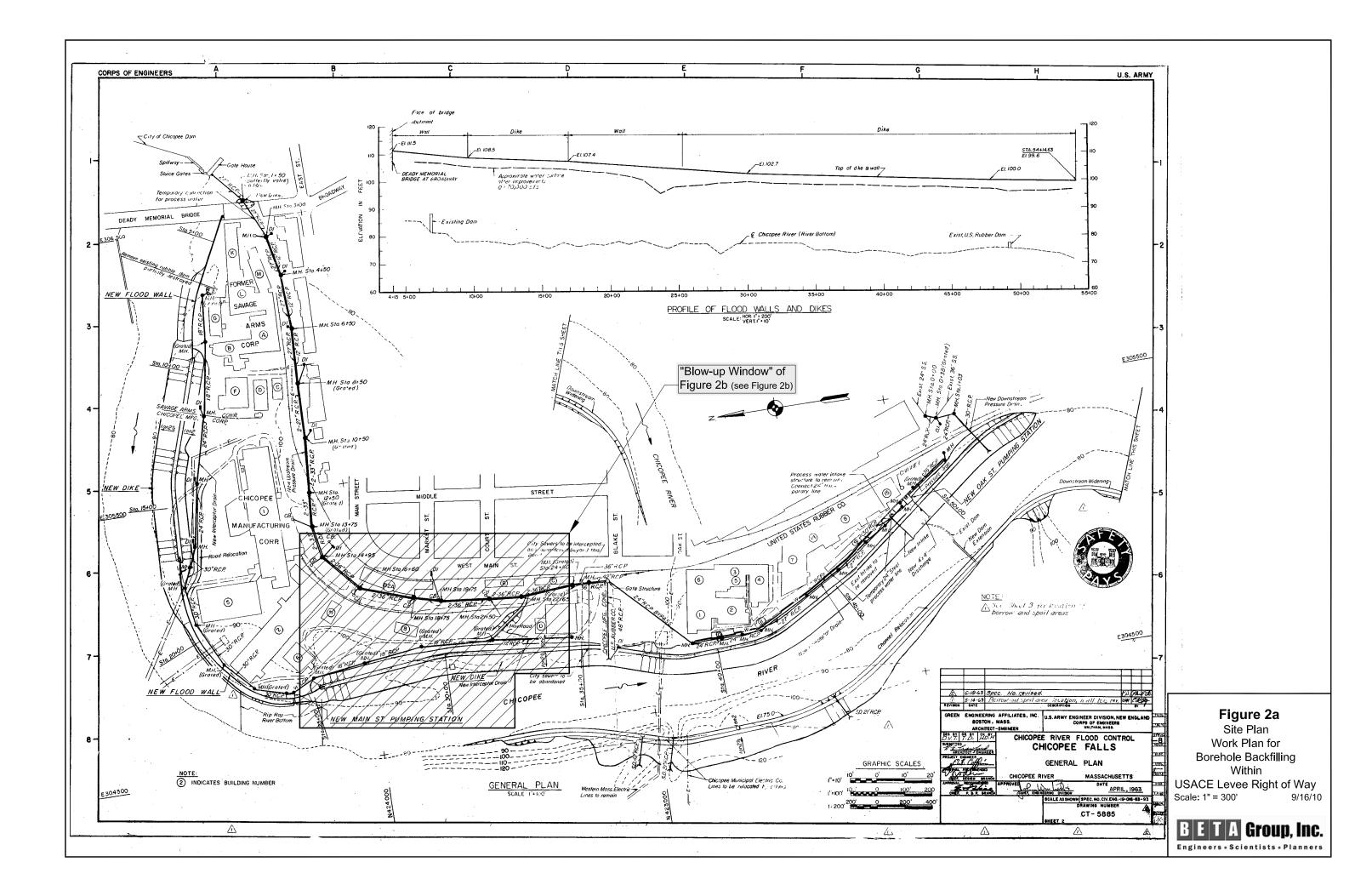
Each work plan will be presented following a brief report format, including a locus figure and a description of the specific proposed activity. When applicable, test holes/test pits will be logged, as will grouting/backfill, and as-built plans will be prepared and submitted to USACE following completion.

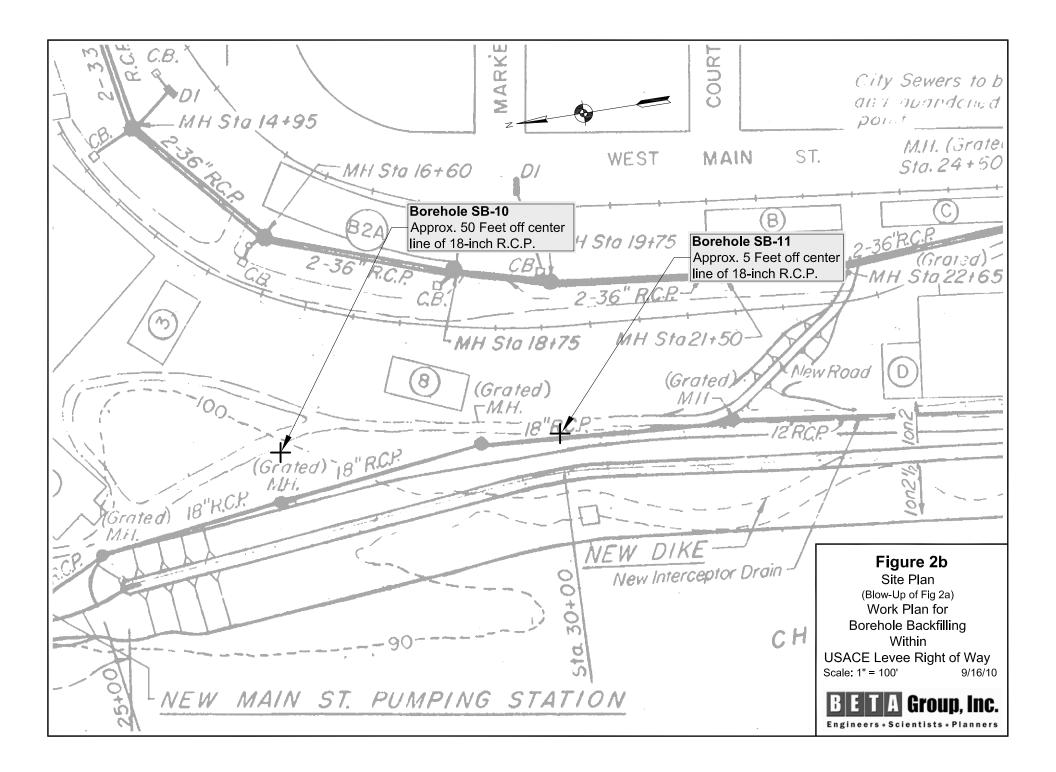
**FIGURES** 



315 Norwood Park South Norwood, MA 02062 781.255.1982 email: BETA@BETA-inc.com Borehole Backfilling Facemate Facility Subsurface Investigations Chicopee Falls, Massachusetts

Locus Plan





# **APPENDIX A**

# **BORING LOGS**

BETA GROUP, INC.							TEST BO	RING RI	EPORT		
PROJECT: Facemate Property - Chicopee						ee	BORING NO.	SB-10			
LOCATION: Chicopee, MA							PAGE 1 OF		1		-
	LING CO:		ical Dri		rvices			DATE STARTED:	8/26/2010		-
EQUIPMENT: HSA								DATE FINISHED:	8/26/2010		_
	LED BY:		James &	' Matt				SURFACE ELEVATION:	na		_
	ECTED BY:		Junes e	e mat				nu		_	
	GROUND		RORSI	TRVAT	TIONS					CORE	
	GROUND		N OD51		10105				CASING	SAMPLER	
	NOT ENG	COUNTE	RED:		-			TYPE:	HSA	Split-Barrel	N/A
	DEPTH	ST	ABILIZA		ME	-		SIZE ID: HAMMER WT:	4-3/8"ID	1-3/8" 140 lb	
		1		None		-		HAMMER FALL:		36"	
						Ĩ	SAMPLE DAT				
DEPTH (ft)	SAMPLING DEPTH		AMMER SAMPLE			STRATA CHANGE		ГНОLOGY tion of materials)	SAMPLE ID	PEN/ RECOV	HNU (ppm)
(11)	FROM - TO	0-6	6-12	12-18	18-24	(ft)	(Desemp	tion of materialsy		(in./in.)	Lamp 10.2 eV.
	0.2						0-6" Topsoil.		0.2	20/24	0.0
	0-2	8	10	44	39			ained brown and coarse angular	0-2	20/24	0.0
_							to rounded gravel.				
	2-4	18	29	27	21	-	0-10" Coarse grained or	ange/brown sand with some	2-4	10/24	0.0
-	21	10	27	27	21		subrounded gravel.			10/21	0.0
5.0	4-6	32	22	19	8		0-6" Fine to medium gra	ined dark brown sand. Some	4-6	6/24	0.0
							subrounded fine gravel.				
_											
	6-8	10	20	20	18		0-2" Same as above.	own sand. Quartz cobble.	6-8	12/24	0.0
							4"-12" Fine grained red				
-				-							
_	8-10	2	6	16	10		0-22" Redish clay with c	oarse angular gravel. Stiff.	8-10	22/24	0.0
10.0						-	2				
1010											
	10-12	4	8	5	6	-	0-18" Same as above.		10-12	18/24	0.0
						-					
_						-					
	12-14	10	22	21	28		0-20" Same as above.		12-14	20/24	0.0
_											
15.0							En	d of Boring			
15.0						-					
_						-					
-						1					
-											
20.0											
20.0		I					1				
١	Notes:	Difficu	ult to tel	ll where	e the w	vater table	e is due to the tight forr	nation.			

BETA GROUP, INC. TEST BORING REPORT											
PROJECT: Facemate Property - Chicopee						ee		BORING NO. SB-11			
LOCATION: Chicopee, MA								PAGE 1 OF	1		
DRILLING CO: Technical Drilling Services								DATE STARTED:	8/26/2010		-
EOUI	PMENT:	HSA		0				DATE FINISHED:	8/26/2010		-
			Iames &	y Matt				SURFACE ELEVATION:	na		-
DRILLED BY: Brett, James & Matt SURFACE ELEVATION INSPECTED BY: SJB									IId		-
111511					TIONS						CODE
GROUNDWATER OBSERVATIONS									CASING	SAMPLER	CORE BARREL
	NOT ENG	COUNTE	RED:					TYPE:	HSA	Split-Barrel	N/A
	DEPTH	ST	ABILIZA		IME	-		SIZE ID:	4-3/8"ID	1-3/8"	
				None		-		HAMMER WT: HAMMER FALL:		140 lb 36"	
							SAMPLE DA'			50	
DEPTH	SAMPLING		AMMER			STRATA		THOLOGY	SAMPLE	PEN/	HNU
(ft)	DEPTH FROM - TO	0-6	SAMPLE 6-12	R (inches 12-18	s) 18-24	CHANGE (ft)	(Descrip	tion of materials)	ID	RECOV (in./in.)	(ppm) Lamp 10.2 eV.
						(11)	0-8" Topsoil.				
	0-2	3	9	10	14	-	8"-20" Fine grained dark	c brown sand and fine angular	0-2	20/24	0.0
							gravel.				
_		_	10	10			0 101 0 1			20/24	
	2-4	7	19	10	9	-	0-10" Same as above. 10"-20" Fine grained ora	ange sand and silt.	2-4	20/24	0.0
_											
5.0	4-6	2	2	2	6				4-6	24/24	0.0
5.0	4-0	2	2	2	0	-	0-24" Fine to coarse gra	ined orange sand. Trace silt.	4-0	24/24	0.0
_											
	6-8	6	4	2	2	-	0-8" Same as above.		6-8	18/24	0.0
						-	8"-18" Medium to coars coarse angular gravel.	se grained brown to red sand and			
_											
_	8-10	19	17	15	19			nge sand and coarse gravel.	8-10	12/24	0.0
10.0						-	6"-12" Red clay with tra	ce coarse sand.			
10.0											
_	10-12	11	20	19	8		0-12" Red clay with son	ne coarse angular gravel. Moist.	10-12	12/24	0.0
						-					
-	12-14	12	18	25	20	-	0-24" Same as above. V	Vet.	12-14	24/24	0.0
						-					
15.0	14.16	22	25	22	24				14.16	10/24	0.0
15.0	14-16	22	25	22	24	-	0-10" Same as above.		14-16	10/24	0.0
_						-					
	16-18	10	25	19	20	-			16-18	20/24	0.0
	10 10	10	23	17	20		0-20" Same as above wi	th a cobble of quartz.	10 10	20/24	0.0
–							End of Boring				
							En				
20.0											
20.0											
Ν	lotes:	Difficu	lt to te	ll wher	e the w	vater tabl	e is due to the tight for	mation.			



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