SHOP DRAWING REVIEW FORM AND TRANSMITTAL

DATE: **October 27, 2021**

TO: Carl Hendrickson
Project Manager
Veolia Water
825 West Water Street
Taunton, MA 02780

FROM: Michael Andrus, P.E.
Project Manager
BETA Group, Inc.
701 George Washington Hwy
Lincoln, Rhode Island 02865

RE: City of Taunton, MA
WWTF Phase 1 Improvements
Contract S-2021-1

Shop Drawing No. 03100 – Loop Ties, Form Release Agent and Epoxy Adhesive

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BETA COMMENTS:

Item	Action Code	Description/Comments
1	**1**	**Loop Ties (Dayton Superior)** **1.** Acceptable as submitted.
2	**1**	**Epoxy Adhesive (Red Head)** **1. Acceptable as Submitted**
3.	**1**	**Form Release Agent (Dayton Superior)** **1. Acceptable as submitted**

Action Codes
1 - No Exception Taken
2 - Make Corrections Noted
3 - Amend and Resubmit
4 - Rejected, See Remarks

a. Installation shall proceed only when Action Code is '1' or '2'.
b. Submittals action coded '3' shall be resubmitted within time limit set in Contract.
c. Review does not relieve Contractor from responsibility of compliance with the Contract Documents.

Hart Engineering Corporation

SUBMITTAL: 03100-01

PROJECT: 9722. - Veolia/Taunton WWTP Solids Handling Improvements **DATE: 10/04/2021**

SUBMITTAL: 03100-01 - Loop Ties, Form Release Agent and Epoxy Adhesive
REVISION: A
STATUS: Eng
SPEC #: 03100

TO:
Carl Hendrickson
Veolia North America
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FROM:
Ryan Murphy
Hart Engineering Corporation
800 Scenic View Drive
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Item	Revision	Description	Status	Date Sent	Date Returned
03100-01	A	Loop Ties, Form Release Agent and Epoxy Adhesive	Eng	10/04/2021	
Notes:					

Additional Notes:

Status Codes
1-APP – No Exceptions Taken
2-ANR – Make C
3-R&R – Revise a
4-REJ – Rejected
5-IPO – For Infor
6-NRR – Not Req
ENG – Submitted

SHOP DRAWING REVIEW

☑ 1 – Approved ☐ 2 – Approved as Noted

☐ 3 – Revise and Resubmit ☐ 4 - Rejected

☐ 5 – Record File Only – No Action Taken

(Above Check Designates Action Code – See Review Comments)

IMPORTANT NOTE FOR CONTRACTOR

Review is only for general compliance with the design concept and information provided in Contract Documents. Corrections and comments made on the Shop Drawings during review do not relieve the Contractor from compliance with the requirements of the plans and specifications. Review and/or approval of a specific item shall not include review or approval of an assembly of which the item is a component. No approval or correction of a Shop Drawing shall be construed as an order for extra work. The Contractor is responsible for: all quantities and dimensions to be confirmed and correlated; information that pertains solely to the fabrication processes or to the means, methods, techniques, sequences and procedures of construction; coordination of the Work with that of all trades and subcontractors; and performing all Work in a safe and satisfactory manner.

BETA GROUP, INC. Checked By: TW

By: BN Date: 10/21/21

Sincerely,
Hart Engineering

10/04/2021

RED HEAD®

AZ+

SUBMITTAL PACKAGE

MAXIMUM VERSATILITY INCREASES ANCHOR RELIABILITY AND JOBSITE PRODUCTIVITY

QUICK-CURE ADHESIVE ANCHOR FOR
CONCRETE AND MASONRY APPLICATIONS

RED HEAD®
ADHESIVE ANCHORING SPECIALISTS

A7+

The Most Versatile Quick Cure Adhesive

A7P-10 A7P-28

APPLICATIONS / USES

- Concrete dowelling (slabs, walls, columns)
- Steel framing (columns, beams, ledgers)
- Brick pinning and CMU reinforcement
- Architectural metal fastening (railings, signage)
- Mechanical, electrical, and plumbing attachment
- Vibratory equipment anchoring

DESCRIPTION

Quick Curing Hybrid Epoxy Adhesive

RED HEAD A7+ is a high-strength, fast-cure adhesive that is designed to securely anchor threaded rod and rebar to cured concrete and masonry. A7+ is one of the most versatile achoring solutions on the market, suitable for use in an extremely wide range of applications and environmental conditions.

- Qualified for use in concrete, brick, block, and clay tile
- ICC-ES approved for cracked concrete and seismic applications (ICC-ES ESR 3903).
- Cures in only 45 minutes (at base temperature of 70°F/21°C)
- No extra time required for drying saturated concrete or water-filled holes
- Easy pumping even in cold temperatures
- Low odor - suitable for use indoors and in occupied buildings
- Optimum viscosity simplifies use in overhead and horizontal holes
- 18-month storage life minimizes waste and risk of using expired product
- Rugged cartirdge resists breakage due to rough handling or cold temperatures

ADVANTAGES

- All weather formula
- Works in damp holes and underwater applications
- Fast curing time, 45 minutes at 70°F
- ICC-ES Evaluation Report ESR-3903 (Concrete) and ESR-3951 (Masonry)
- NSF 61 Listed
- S75 high flow nozzle reduces installation time
- Fast & easy dispensing, even 28 ounce cartridge can be hand dispensed

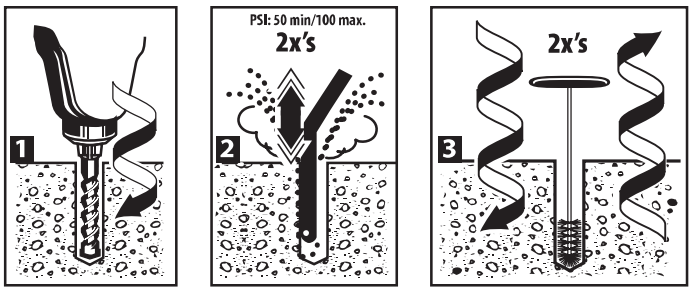
Curing Times

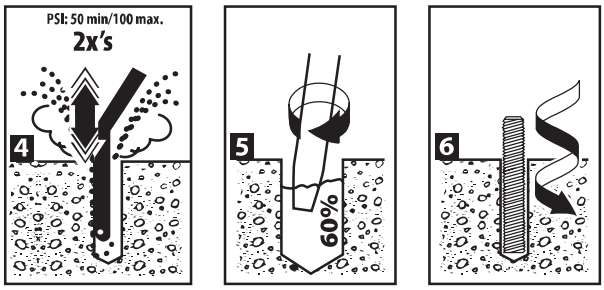
CONCRETE (F°)	ADHESIVE (F°)	GEL TIME	FULL CURE TIME
110	110	1.5 minutes	45 minutes
90	90	3 minutes	45 minutes
70	70	5 minutes	45 minutes
50	50	15 minutes	90 minutes
30	30	35 minutes	4 hours
14	30	35 minutes	24 hours

Most Competitive Spacing and Edge Distance

NOMINAL ANCHOR DIAMETER (IN.)	MINIMUM SPACING (IN.)	MINIMUM EDGE DISTANCE (IN.)
3/8	15/16	15/16
1/2	1-1/2	1-1/2
5/8	2-1/2	2-1/2
3/4	3	3
7/8	3-1/2	3-1/2
1	4	4
1-1/4	5	5

INSTALLATION STEPS





* Damp, submerged and underwater applications require 4x's air, 4x's brushing and 4x's air

APPROVALS/LISTINGS

ICC-ES ESR-3903 for Cracked and Uncracked concrete including all Seismic Zones

ICC-ES ESR-3951 for masonry

IBC 2006/2009/2012/2015 Compliant

NSF/ANSI Standard 61

For the most current approvals/listings visit: www.itwredhead.com

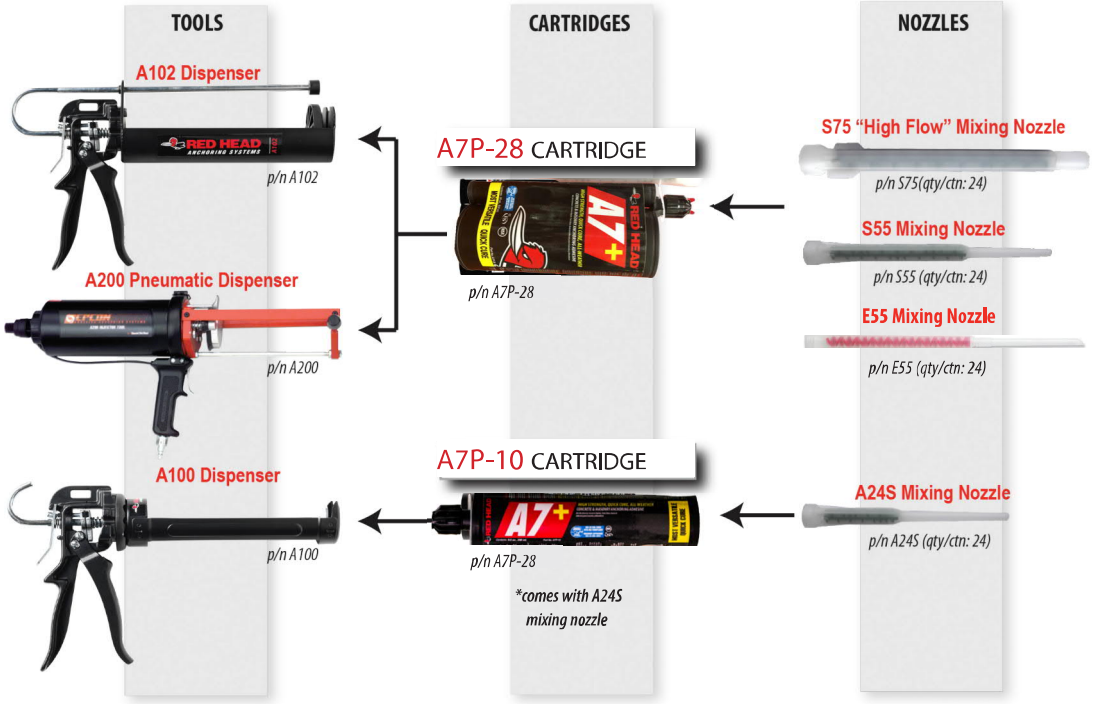
APPLICATIONS

Water Treatment Facilities

The best-in-class in edge and spacing distance of Red Head A7+ and its ability to work in water have make it a great fit for waste water treatment plants.

Roadway Doweling

A7+ dispenses so quickly and rebar inserts so easily that contractors find installed costs are lower than many other products including grout for doweling.



A7P–28 fl. oz. Ordering Information

PART NUMBER	DESCRIPTION	BOX QTY
 A7P-28	28 Fluid Ounce Cartridge A7+ Each cartirdge comes with a S55 Nozzle	4
 E55	Mixing Nozzle for A7P-28 and G5-22 Cartridge Nozzle diameter fits 3/8" to 5/8" holes. (overall length of nozzle 14")	24
 A102	*Largest hand dispensable cartridge—still easy to dispense* Hand Dispenser for A7P-28 Cartridge	1

PART NUMBER	DESCRIPTION	BOX QTY
E25-6	6-Foot Straight Tubing (Used when holes are deeper) (can cut to proper size) (.39 in I.D. x .43 in. O.D.)	6
 A200	Pneumatic Dispenser for A7P-28 Cartridge	1
 S55	Mixing Nozzle for A7P-28 Cartridge Nozzle diameter fits holes for 3/8" diameter & larger anchors (overall length of nozzle 10")	24

ESTIMATING TABLE

A7+
28 Fluid Ounce Cartridge

Number of Anchoring Installations per Cartridge* Using Reinforcing Bar with A7+ Adhesive in Solid Concrete

REBAR	DRILL HOLE DIA. INCHES	EMBEDMENT DEPTH IN INCHES (mm)														
		1 (25.4)	2 (50.8)	3 (76.2)	4 (101.6)	5 (127.0)	6 (152.4)	7 (177.8)	8 (203.2)	9 (228.6)	10 (254.0)	11 (279.4)	12 (304.8)	13 (330.2)	14 (355.6)	15 (381.0)
# 3	7/16	662.5	331.3	220.8	165.6	132.5	110.4	94.6	82.8	73.6	66.3	60.2	55.2	51.0	47.3	44.2
# 4	5/8	373.0	186.5	124.3	93.2	74.6	62.2	53.3	46.6	41.4	37.3	33.9	31.1	28.7	26.6	24.9
# 5	3/4	286.1	143.0	95.4	71.5	57.2	47.7	40.9	35.8	31.8	28.6	26.0	23.8	22.0	20.4	19.1
# 6	7/8	231.0	115.5	77.0	57.7	46.2	38.5	33.3	28.8	25.7	23.1	21.0	19.2	17.8	16.5	15.4
# 7	1	213.4	106.7	71.1	53.3	42.7	35.6	30.5	26.7	23.7	21.3	19.4	17.8	16.4	15.2	14.2
# 8	1-1/8	177.3	88.6	59.1	44.3	35.5	29.5	25.3	22.2	19.7	17.7	16.1	14.8	13.6	12.7	11.8
# 9	1-1/4	102.8	51.4	34.3	25.7	20.6	17.1	14.7	12.8	11.4	10.3	9.3	8.6	7.9	7.3	6.9
# 10	1-1/2	84.1	42.0	28.0	21.0	16.8	14.0	12.0	10.5	9.3	8.4	7.6	7.0	6.5	6.0	5.6
# 11	1-3/4	51.4	25.7	17.1	12.8	10.3	8.6	7.3	6.4	5.7	5.1	4.7	4.3	4.0	3.7	3.4

* The number of anchoring installations is based upon calculations of hole volumes using ANSI tolerance carbide tipped drill bits, the nominal areas of the reinforcing bars and the stress areas of the threaded rods. These estimates do not account for waste.

ESTIMATING TABLE

A7+
28 Fluid Ounce Cartridge

Number of Anchoring Installations per Cartridge* Using Threaded Rod with A7+ Adhesive in Solid Concrete

ROD In. (mm)	DRILL HOLE DIA. INCHES	EMBEDMENT DEPTH IN INCHES (mm)														
		1 (25.4)	2 (50.8)	3 (76.2)	4 (101.6)	5 (127.0)	6 (152.4)	7 (177.8)	8 (203.2)	9 (228.6)	10 (254.0)	11 (279.4)	12 (304.8)	13 (330.2)	14 (355.6)	15 (381.0)
1/4 (6.4)	5/16	915.5	457.7	305.2	228.9	183.1	152.8	130.8	114.4	101.7	91.5	83.2	76.3	70.4	65.4	61.0
3/8 (9.5)	7/16	530.0	265.0	176.7	132.5	106.0	88.3	75.7	66.3	58.9	53.0	48.2	44.2	40.8	37.9	35.3
1/2 (12.7)	9/16	381.4	190.7	127.1	95.4	76.3	63.6	54.5	47.7	42.4	38.1	34.7	31.8	29.3	27.2	25.4
5/8 (15.9)	3/4	195.6	97.8	65.1	48.8	39.0	32.5	27.9	24.4	21.7	19.5	17.7	16.3	15.0	13.9	13.0
3/4 (19.1)	7/8	154.4	77.2	51.5	38.6	30.9	25.7	22.1	19.3	17.2	15.4	14.0	12.9	11.9	11.0	10.3
7/8 (22.2)	1	128.0	64.0	42.8	32.0	25.6	21.4	18.3	16.0	14.2	12.8	11.6	10.7	9.9	9.2	8.5
1 (25.4)	1 -1/8	105.2	52.6	35.2	26.3	21.1	17.6	15.0	13.2	11.7	10.5	9.6	8.8	8.1	7.6	7.0
1-1/4 (31.8)	1 -3/8	80.0	40.0	26.6	20.0	15.9	13.3	11.4	10.0	8.9	8.0	7.2	6.6	6.1	5.7	5.3

* The number of anchoring installations is based upon calculations of hole volumes using ANSI tolerance carbide tipped drill bits, the nominal areas of the reinforcing bars and the stress areas of the threaded rods. These estimates do not account for waste.

A7P–10 fl. oz. Ordering Information

PART NUMBER	DESCRIPTION	BOX QTY
 A7P-10	9.5 Fluid Ounce Cartridge with Nozzle	6
 A24S	Mixing Nozzle for A7P-10 Cartridge Nozzle diameter fits 3/8" to 5/8" holes (overall length of nozzle 6-3/8")	24
 A100	Hand Dispenser Designed for A7P-10 Cartridge Contractor Quality 26:1 Thrust Ratio	1

PACKAGING

1. Disposable, self-contained cartridge system capable of dispensing both components in the proper mixing ratio
2. Acrylic components dispensed through a static mixing nozzle that thoroughly mixes the material and places the material at the base of the pre-drilled hole
3. Cartridge markings: Include manufacturer's name, batch number and best-used-by date, mix ratio by volume, ANSI hazard classification, and appropriate ANSI handling precautions

SUGGESTED SPECIFICATIONS

ACRYLIC ADHESIVE:

1. Meets NSF Standard 61, certified for use in conjunction with drinking water systems.
2. Works in wet, damp, submerged holes.
3. Shelf life: Best if used within 18 months.
4. All weather, cure time (45 min. at 70°F).
5. Dispenses easier and faster.
6. Dispenses and cures faster in cold weather, alsoworks in hot weather.
7. Pumpable at 32°F without preheating.
8. Formula for use in solid and hollow base materials.
9. Suitable for oversized and diamond cored holes with increased depths.
10. Quick insertion time = less labor cost.

ESTIMATING TABLES

A7+ 9.5 Fluid Ounce Cartridge

Number of Anchoring Installations per Cartridge* Using Reinforcing Bar and Threaded Rod with A7+ Adhesive in Solid Concrete

REBAR	DRILL HOLE DIA. INCHES	EMBEDMENT DEPTH IN INCHES (mm)			
		2 (50.8)	4 (101.6)	6 (152.4)	8 (203.2)
# 3	7/16	110	55	37	27
# 4	5/8	63	31	20	14
# 5	3/4	48	24	16	11
# 6	7/8	39	18	13	9
# 7	1	35	18	11	9
# 8	1-1/8	29	14	9	7

ROD In (mm)	DRILL HOLE DIA. INCHES	EMBEDMENT DEPTH IN INCHES (mm)				
		2 (50.8)	4 (101.6)	6 (152.4)	8 (203.2)	10 (254.0)
3/8 (9.5)	7/16	88	44	28	22	18
1/2 (12.7)	9/16	65	31	22	16	13
5/8 (15.9)	3/4	33	16	11	7	6.5
3/4 (19.1)	7/8	26	13	9	7	5
7/8 (22.2)	1	22	11	7	5	4.5
1 (25.4)	1-1/8	18	9	5	3	3.5

* The number of anchoring installations is based upon calculations of hole volumes using ANSI tolerance carbide tipped drill bits, the nominal areas of the reinforcing bars and the stress areas of the threaded rods. These estimates do not account for waste.

A7P–5 fl. oz. Ordering Information

PART NUMBER	DESCRIPTION	BOX QTY
 A7P-500KIT	Convenient Dispensing Kit Packaged in a Solid Plastic Shell with (1) A500 Plastic Dispenser (1) A7P-5 Cartridge and (1) A24 Nozzle Nozzle diameter fits 3/8" to 5/8" holes	8

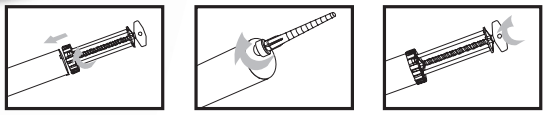
PART NUMBER	DESCRIPTION	BOX QTY
 A7P-501KIT	Convenient Dispensing Kit Packaged in a Solid Plastic Shell with (1) A501 Plastic Dispenser (1) A7P-5 Cartridge and (1) A24 Nozzle Nozzle diameter fits 3/8" to 5/8" holes	8

AVAILABLE WITH YOUR CHOICE OF TWO, EASY DISPENSING SYSTEMS

A500 PLASTIC DISPENSER

Attaches directly to cartridge allowing for easy hand dispensing. **No extra tools are required.**

Simple Assembly and Dispensing

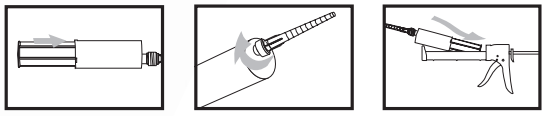


1. Twist-lock dispenser onto cartridge.
2. Thread nozzle onto cartridge.
3. Turn lever in order to dispense adhesive.

A501 CAULKING GUN ADAPTOR

Allows cartridge to work with most standard caulking guns (caulking gun supplied by contractor).

Simple Assembly and Dispensing



1. Push adaptor tightly against back of cartridge.
2. Thread nozzle onto cartridge.
3. Place assembly in caulking gun and dispense adhesive.

EASY PACKAGING!

A500 and A501 kits are perfect for both counter or pegboard hanging display.

A7P-500KIT

A7P-501KIT

ESTIMATING TABLES

A7+ 5 Fluid Ounce Cartridge

Number of Anchoring Installations per Cartridge Using Reinforcing Bar and Threaded Rod with A7+ Adhesive in Solid Concrete*

REBAR	DRILL HOLE DIA. INCHES	EMBEDMENT DEPTH IN INCHES (mm)			
		2 (50.8)	4 (101.6)	6 (152.4)	8 (203.2)
# 3	7/16	60	30	20	15
# 4	5/8	34	17	11	8
# 5	3/4	26	13	9	6
# 6	7/8	21	10	7	5
# 7	1	19	10	6	5
# 8	1-1/8	16	8	5	4

ROD In (mm)	DRILL HOLE DIA. INCHES	EMBEDMENT DEPTH IN INCHES (mm)			
		2 (50.8)	4 (101.6)	6 (152.4)	8 (203.2)
3/8 (9.5)	7/16	48	24	16	12
1/2 (12.7)	9/16	35	17	12	9
5/8 (15.9)	3/4	18	9	6	4
3/4 (19.1)	7/8	14	7	5	4
7/8 (22.2)	1	12	6	4	3
1 (25.4)	1-1/8	10	5	3	2

* The number of anchoring installations is based upon calculations of hole volumes using ANSI tolerance carbide tipped drill bits, the nominal areas of the reinforcing bars and the stress areas of the threaded rods. These estimates do not account for waste.

PERFORMANCE TABLE

A7+ Quick-Cure Adhesive

Average Ultimate Tension and Shear Loads [1,2,3] for Threaded Rod Installed in Solid Concrete

THREADED ROD DIA. In. (mm)	DRILL HOLE DIAMETER In. (mm)	MAX. CLAMPING FORCE AFTER PROPER CURE Ft.-Lbs. (Nm)	EMBEDMENT IN CONCRETE In. (mm)	2000 PSI (13.8 MPa) CONCRETE		4000 PSI (27.6 MPa) CONCRETE	
				ULTIMATE TENSION Lbs. (kN)	ULTIMATE SHEAR Lbs. (kN)	ULTIMATE TENSION Lbs. (kN)	ULTIMATE SHEAR Lbs. (kN)
3/8 (9.5)	7/16 (11.1)	13 - 18 (17-24)	1-1/2 (38.1)	N/A	N/A	3,734 (16.6)	4,126 (18.3)
			3-3/8 (85.7)	5,852 (26.0)	5,220 (23.2)	10,977 (48.8)	5,220 (23.2)
			4-1/2 (114.3)	7,729 (34.4)	5,220 (23.2)	11,661 (51.9)	5,220 (23.2)
1/2 (12.7)	9/16 (14.3)	22 - 25 (29-33)	2 (50.8)	N/A	N/A	6,022 (26.8)	8,029 (35.7)
			4-1/2 (114.3)	10,798 (48.0)	8,029 (35.7)	17,162 (76.3)	8,029 (35.7)
			6 (152.4)	14,210 (63.2)	8,029 (35.7)	17,372 (77.3)	8,029 (35.7)
5/8 (15.9)	3/4 (19.1)	55 - 80 (74-108)	2-1/2 (63.5)	N/A	N/A	7,330 (32.6)	11,256 (50.1)
			5-5/8 (142.9)	16,417 (73.0)	15,967 (71.0)	26,504 (117.9)	15,967 (71.0)
			7-1/2 (190.5)	18,747 (83.4)	15,967 (71.0)	29,381 (130.7)	15,967 (71.0)
3/4 (19.1)	7/8 (22.2)	106 - 160 (143-216)	3 (76.2)	N/A	N/A	8,634 (38.4)	20,126 (89.5)
			6-3/4 (171.5)	18,618 (82.8)	20,126 (89.5)	29,727 (132.2)	20,126 (89.5)
			9 (228.6)	23,934 (106.5)	20,126 (89.5)	37,728 (167.8)	20,126 (89.5)
7/8 (22.2)	1 (25.4)	185 - 250 (250-338)	3-1/2 (88.9)	N/A	N/A	13,650 (60.7)	20,920 (92.9)
			7-7/8 (200.0)	N/A	29,866 (132.9)	44,915 (199.8)	29,866 (132.9)
			10-1/2 (266.7)	36,881 (164.1)	29,866 (132.9)	48,321 (215.0)	29,866 (132.9)
1 (25.4)	1-1/8 (28.6)	276 - 330 (374-447)	4 (101.6)	N/A	N/A	16,266 (72.2)	33,152 (147.5)
			9 (228.6)	32,215 (143.3)	37,538 (167.0)	48,209 (214.5)	37,538 (167.0)
			12 (304.8)	46,064 (204.9)	37,538 (167.0)	63,950 (284.5)	37,538 (167.0)
1-1/4 (31.8)	1-3/8 (34.9)	370 - 660 (501-894)	5 (127.0)	N/A	N/A	21,838 (97.1)	33,152 (147.5)
			11-1/4 (285.8)	45,962 (204.5)	58,412 (259.8)	56,715 (252.3)	58,412 (259.8)
			15 (381.0)	62,208 (276.7)	58,412 (259.8)	84,385 (375.4)	58,412 (259.8)

1 Allowable working loads for the single installations under static loading should not exceed 25% capacity or the allowable load of the anchor rod. Divide by 4.

2 Ultimate load values in 2000 and 4000 psi stone aggregate concrete. Ultimate loads are indicated for the embedment shown in the Embedment in Concrete column. Performance values are based on the use of high strength threaded rod (ASTM A193 Gr. B7). The use of lower strength rods will result in lower ultimate tension and shear loads.

3 Linear interpolation may be used for intermediate spacing and edge distances.

PERFORMANCE TABLE

A7+ Quick-Cure Adhesive

Allowable Tension Loads [1] for Threaded Rod Installed in Solid Concrete

THREADED ROD DIA. In. (mm)	DRILL HOLE DIAMETER In. (mm)	MIN. EMBEDMENT DEPTH In. (mm)	ALLOWABLE TENSION LOAD BASED ON ADHESIVE BOND STRENGTH		ALLOWABLE TENSION LOAD BASED ON STEEL STRENGTH		
			2000 PSI (13.8 MPa) CONCRETE Lbs. (kN)	4000 PSI (27.6 MPa) CONCRETE Lbs. (kN)	ASTM A307 (SAE 1018) Lbs. (kN)	ASTM A193 GR. B7 (SAE 4140) Lbs. (kN)	ASTM F593 AISI 304 SS Lbs. (kN)
3/8 (9.5)	7/16 (11.1)	1-1/2 (38.1)	N/A	934 (4.2)	2,080 (9.3)	4,340 (19.3)	3,995 (17.8)
		3-3/8 (85.7)	1,460 (6.5)	2,740 (12.2)	2,080 (9.3)	4,340 (19.3)	3,995 (17.8)
		4-1/2 (114.3)	1,930 (8.6)	2,915 (13.0)	2,080 (9.3)	4,340 (19.3)	3,995 (17.8)
1/2 (12.7)	9/16 (14.3)	2 (50.8)	N/A	1,505 (6.7)	3,730 (16.6)	7,780 (34.6)	7,155 (31.8)
		4-1/2 (114.3)	2,700 (12.0)	4,290 (19.1)	3,730 (16.6)	7,780 (34.6)	7,155 (31.8)
		6 (152.4)	3,550 (15.8)	4,340 (19.3)	3,730 (16.6)	7,780 (34.6)	7,155 (31.8)
5/8 (15.9)	3/4 (19.1)	2-1/2 (63.5)	N/A	1,832 (8.2)	5,870 (26.1)	12,230 (54.4)	11,250 (50.0)
		5-5/8 (142.9)	4,100 (18.3)	6,625 (29.5)	5,870 (26.1)	12,230 (54.4)	11,250 (50.0)
		7-1/2 (190.5)	4,685 (20.8)	7,345 (32.7)	5,870 (26.1)	12,230 (54.4)	11,250 (50.0)
3/4 (19.1)	7/8 (22.2)	3 (76.2)	N/A	2,158 (9.6)	8,490 (37.8)	17,690 (78.7)	14,860 (66.1)
		6-3/4 (171.5)	4,655 (20.7)	7,430 (33.1)	8,490 (37.8)	17,690 (78.7)	14,860 (66.1)
		9 (228.6)	5,980 (26.6)	9,430 (42.0)	8,490 (37.8)	17,690 (78.7)	14,860 (66.1)
7/8 (22.2)	1 (25.4)	3-1/2 (88.9)	N/A	3,413 (15.2)	11,600 (51.6)	25,510 (113.5)	20,835 (92.7)
		7-7/8 (200.0)	N/A	11,230 (49.9)	11,600 (51.6)	25,510 (113.5)	20,835 (92.7)
		10-1/2 (266.7)	9,220 (41.0)	12,080 (53.7)	11,600 (51.6)	25,510 (113.5)	20,834 (92.7)
1 (25.4)	1-1/8 (28.6)	4 (101.6)	N/A	4,067 (18.1)	15,180 (67.5)	31,620 (140.7)	26,560 (118.1)
		9 (228.6)	8,050 (35.8)	12,050 (53.6)	15,180 (67.5)	31,620 (140.7)	26,560 (118.1)
		12 (304.8)	11,515 (51.2)	15,985 (71.1)	15,180 (67.5)	31,620 (140.7)	26,560 (118.1)
1-1/4 (31.8)	1-3/8 (34.9)	5 (127.0)	N/A	5,460 (24.3)	23,800 (105.9)	49,580 (220.6)	34,670 (154.2)
		11-1/4 (285.8)	11,490 (51.1)	14,175 (63.1)	23,800 (105.9)	49,580 (220.6)	34,670 (154.2)
		15 (381.0)	15,550 (69.2)	21,095 (93.8)	23,800 (105.9)	49,580 (220.6)	34,670 (154.2)

1 Use lower value of either bond or steel strength for allowable tensile load.

PERFORMANCE TABLE

A7+ Quick-Cure Adhesive

Allowable Shear Loads[1] for Threaded Rod Installed in Solid Concrete

THREADED ROD DIA. In. (mm)	DRILL HOLE DIAMETER In. (mm)	MIN. EMBEDMENT DEPTH In. (mm)	ALLOWABLE SHEAR LOAD BASED ON CONCRETE STRENGTH		ALLOWABLE SHEAR LOAD BASED ON STEEL STRENGTH		
			2000 PSI (13.8 MPa) CONCRETE Lbs. (kN)	4000 PSI (27.6 MPa) CONCRETE Lbs. (kN)	ASTM A307 (SAE 1018) Lbs. (kN)	ASTM A193 GR. B7 (SAE 4140) Lbs. (kN)	ASTM F593 AISI 304 SS Lbs. (kN)
3/8 (9.5)	7/16 (11.1)	1-1/2 (38.1)	N/A	1,031 (4.6)	1,040 (4.6)	2,170 (9.7)	1,995 (8.9)
		3-3/8 (85.7)	1,305 (5.8)	1,305 (5.8)	1,040 (4.6)	2,170 (9.7)	1,995 (8.9)
1/2 (12.7)	9/16 (14.3)	2 (50.8)	N/A	2,005 (8.9)	1,870 (8.3)	3,895 (17.3)	3,585 (15.9)
		4-1/2 (114.3)	2,005 (8.9)	2,005 (8.9)	1,870 (8.3)	3,895 (17.3)	3,585 (15.9)
5/8 (15.9)	3/4 (19.1)	2-1/2 (63.5)	N/A	2,814 (12.5)	2,940 (13.1)	6,125 (27.2)	5,635 (25.1)
		5-5/8 (142.9)	3,990 (17.8)	3,990 (17.8)	2,940 (13.1)	6,125 (27.2)	5,635 (25.1)
3/4 (19.1)	7/8 (22.2)	3 (76.2)	N/A	5,030 (22.4)	4,250 (18.9)	8,855 (39.4)	7,440 (33.1)
		6-3/4 (171.5)	5,030 (22.4)	5,030 (22.4)	4,250 (18.9)	8,855 (39.4)	7,440 (33.1)
7/8 (22.2)	1 (25.4)	3-1/2 (88.9)	N/A	5,230 (23.3)	5,800 (25.8)	12,760 (56.8)	10,730 (47.7)
		7-7/8 (200.0)	7,465 (33.2)	7,465 (33.2)	5,800 (25.8)	12,760 (56.8)	10,730 (47.7)
1 (25.4)	1-1/8 (28.6)	4 (101.6)	N/A	8,288 (36.9)	7,590 (33.8)	15,810 (70.3)	13,285 (59.1)
		9 (228.6)	9,385 (41.7)	9,385 (41.7)	7,590 (33.8)	15,810 (70.3)	13,285 (59.1)
1-1/4 (31.8)	1-3/8 (34.9)	5 (127.0)	N/A	8,288 (36.9)	11,900 (52.9)	24,790 (100.3)	18,840 (83.8)
		11-1/4 (285.8)	14,600 (64.9)	14,600 (64.9)	11,900 (52.9)	24,790 (100.3)	18,840 (83.8)

1 Use lower value of either concrete or steel strength for allowable shear load.

PERFORMANCE TABLE

A7+ Quick-Cure Adhesive

Average Ultimate Tension Loads[1,2,3] for Reinforcing Bar Installed in Solid Concrete

REINFORCING BAR DIA. In. (mm)	EMBEDMENT IN CONCRETE In. (mm)	2000 PSI (13.8 MPa) CONCRETE ULTIMATE TENSION Lbs. (kN)	4000 PSI (27.6 MPa) CONCRETE ULTIMATE TENSION Lbs. (kN)	ULTIMATE TENSILE AND YIELD STRENGTH GRADE 60 REBAR	
				MINIMUM YIELD STRENGTH Lbs. (kN)	MINIMUM ULTIMATE TENSILE STRENGTH Lbs. (kN)
# 3 (9.5)	3-3/8 (85.7)	6,180 (27.5)	8,324 (37.0)	6,600 (29.4)	9,900 (44.0)
	4-1/2 (114.3)	7,560 (33.6)	11,418 (50.8)	6,600 (29.4)	9,900 (44.0)
# 4 (12.7)	4-1/2 (114.3)	9,949 (44.3)	16,657 (74.1)	12,000 (53.4)	18,000 (80.1)
	6 (152.4)	15,038 (66.9)	17,828 (79.3)	12,000 (53.4)	18,000 (80.1)
# 5 (15.9)	5-5/8 (142.9)	14,012 (62.3)	20,896 (93.0)	18,600 (82.7)	27,900 (124.1)
	7-1/2 (190.5)	16,718 (74.4)	26,072 (116.0)	18,600 (82.7)	27,900 (124.1)
# 6 (19.1)	6-3/4 (171.5)	21,247 (94.5)	26,691 (118.7)	26,400 (117.4)	39,600 (176.2)
	9 (228.6)	33,325 (148.2)	37,425 (166.5)	26,400 (117.4)	39,600 (176.2)
# 7 (22.2)	7-7/8 (200.0)	N/A	40,374 (179.6)	36,000 (160.1)	54,000 (240.2)
	10-1/2 (266.7)	38,975 (173.4)	46,050 (204.8)	36,000 (160.1)	54,000 (240.2)
# 8 (25.4)	9 (228.6)	35,600 (158.4)	47,311 (210.5)	47,400 (210.9)	71,100 (316.3)
	12 (304.8)	41,010 (182.4)	66,140 (294.2)	47,400 (210.9)	71,100 (316.3)
# 9 (28.6)	10-1/8 (257.2)	N/A	57,221 (254.5)	60,000 (266.9)	90,000 (400.4)
	13-1/2 (342.9)	N/A	79,966 (355.7)	60,000 (266.9)	90,000 (400.4)
# 10 (31.8)	11-1/4 (285.8)	49,045 (218.2)	73,091 (325.1)	76,200 (339.0)	114,300 (508.5)
	15 (381.0)	69,079 (307.3)	83,295 (370.5)	76,200 (339.0)	114,300 (508.5)
# 11 (34.9)	12-3/8 (314.3)	63,397 (282.0)	75,047 (333.8)	93,600 (416.4)	140,400 (624.6)
	16-1/2 (419.1)	81,707 (363.5)	91,989 (409.2)	93,600 (416.4)	140,400 (624.6)

1 Allowable working loads for the single installations under static loading should not exceed 25% capacity or the allowable load of the anchor rod.

2 Ultimate load values in 2000 and 4000 psi stone aggregate concrete. Ultimate loads are indicated for the embedment shown in the Embedment in Concrete column. Performance values are based on the use of minimum Grade 60 reinforcing bar. The use of lower strength rods will result in lower ultimate tension loads.

3 SHEAR DATA: Provided the distance from the rebar to the edge of the concrete member exceeds 1.25 times the embedment depth of the rebar, calculate the ultimate shear load for the rebar anchorage as 60% of the ultimate tensile strength of the rebar.

PERFORMANCE TABLE

A7+ Quick-Cure Adhesive

Recommended Edge Distance Requirements for Shear Loads Installed in Solid Concrete

ANCHOR DIAMETER In. (mm)	EMBEDMENT DEPTH In. (mm)	CRITICAL EDGE DISTANCE In. (mm) 100% LOAD CAPACITY	INTERPOLATED EDGE DISTANCE In. (mm) (80% LOAD CAPACITY)	INTERPOLATED EDGE DISTANCE In. (mm) (50% LOAD CAPACITY)	MINIMUM EDGE DISTANCE In. (mm) (10% LOAD CAPACITY)
3/8 (9.5)	3-3/8 (85.7)	4-3/16 (106.4)	3-7/16 (87.3)	2-5/16 (58.7)	13/16 (20.6)
1/2 (12.7)	4-1/2 (114.3)	5-5/8 (142.9)	4-5/8 (117.5)	3-1/8 (79.4)	1-1/8 (28.6)
5/8 (15.9)	5-5/8 (142.9)	7 (177.8)	5-3/4 (146.1)	3-1/8 (79.4)	1-3/8 (34.9)
3/4 (19.1)	6-3/4 (171.5)	8-7/16 (214.2)	6-15/16 (176.2)	4-5/8 (117.5)	1-5/8 (41.3)
1 (25.4)	9 (228.6)	11-1/4 (285.8)	9-1/4 (235.0)	6-1/4 (158.8)	2-1/4 (57.2)
1-1/4 (31.8)	11-1/4 (285.8)	14-1/16 (357.2)	11-5/8 (295.3)	7-7/8 (200.0)	2-7/8 (73.0)

PERFORMANCE TABLE

A7+ Quick-Cure Adhesive

Recommended Edge Distance Requirements for Tension Loads Installed in Solid Concrete

ANCHOR DIAMETER In. (mm)	EMBEDMENT DEPTH In. (mm)	CRITICAL EDGE DISTANCE In. (mm) (100% LOAD CAPACITY)	INTERPOLATED EDGE DISTANCE In. (mm) (90% LOAD CAPACITY)	INTERPOLATED EDGE DISTANCE In. (mm) (80% LOAD CAPACITY)	MINIMUM EDGE DISTANCE In. (mm) (70% LOAD CAPACITY)
3/8 (9.5)	3-3/8 (85.7) 4-1/2 (114.3)	2-1/2 (63.5) 3-3/8 (85.7)	1-15/16 (49.2) 2-5/8 (66.7)	1-3/8 (34.9) 1-7/8 (47.6)	13/16 (26.2) 1-1/8 (28.6)
1/2 (12.7)	4-1/2 (114.3) 6 (152.4)	3-3/8 (85.7) 4-1/2 (114.3)	2-5/8 (66.7) 3-1/2 (88.9)	1-7/8 (47.6) 2-1/2 (63.5)	1-1/8 (28.6) 1-1/2 (38.1)
5/8 (15.9)	5-5/8 (142.9) 7-1/2 (190.5)	4-3/16 (106.4) 5-5/8 (142.9)	3-1/4 (82.6) 4-3/8 (111.1)	2-5/16 (58.7) 3-1/8 (79.4)	1-3/8 (34.9) 1-7/8 (47.6)
3/4 (19.1)	6-3/4 (171.5) 9 (228.6)	5-1/16 (128.6) 6-3/4 (171.5)	3-15/16 (100.0) 5-1/4 (133.4)	2-13/16 (71.4) 3-3/4 (95.3)	1-5/8 (15.9) 2-1/4 (57.2)
1 (25.4)	9 (228.6) 12 (304.8)	6-3/4 (171.5) 9 (228.6)	5-1/4 (133.4) 7 (177.8)	3-3/4 (95.3) 5 (127.0)	2-1/4 (57.2) 3 (76.2)
1-1/4 (31.8)	11-1/4 (285.8) 15 (381.0)	8-7/16 (214.3) 11-1/4 (285.8)	6-9/16 (166.7) 8-3/4 (222.2)	4-3/4 (120.7) 6-1/4 158.8)	2-7/8 (73.0) 3-3/4 (95.3)

A7+ Quick-Cure Adhesive

Allowable Stress Design Reference Tables

A7+ Adhesive Edge/Spacing Distance Load Factor Summary for Installation of Threaded Rod and Reinforcing Bar [1, 2]

LOAD FACTOR	DISTANCE FROM EDGE OF CONCRETE
Critical Edge Distance—Tension	
100% Tension Load	0.75 x Anchor Embedment
Minimum Edge Distance—Tension	
70% Tension Load	0.25 x Anchor Embedment
Critical Edge Distance—Shear	
100% Shear Load	1.25 x Anchor Embedment
Minimum Edge Distance—Shear	
10% Shear Load	0.25 x Anchor Embedment

LOAD FACTOR	DISTANCE FROM ANOTHER ANCHOR
Critical Spacing—Tension	
100% Tension Load	1.25 x Anchor Embedment
Minimum Spacing—Tension	
80% Tension Load	0.25 x Anchor Embedment
Critical Spacing—Shear	
100% Shear Load	1.25 x Anchor Embedment
Minimum Spacing—Shear	
25% Shear Load	0.25 x Anchor Embedment

1 Use linear interpolation for load factors at edge distances or spacing distances between critical and minimum.

2 Anchors are affected by multiple combination of spacing and/or edge distance loading and direction of the loading. Use the product of tension and shear loading factors in design.

Combined Tension and Shear Loading—for A7+ Adhesive Anchors

Allowable loads for anchors under tension and shear loading at the same time (combined loading) will be lower than the allowable loads for anchors subjected to 100% tension or 100% shear. Use the following equation to evaluate anchors in combined loading conditions:

$$\left(\frac{Na}{Ns}\right)^{5/3} + \left(\frac{Va}{Vs}\right)^{5/3} \leq 1$$

Na = **Applied Service Tension Load**

Ns = **Allowable Tension Load**

Va = **Applied Service Shear Load**

Vs = **Allowable Shear Load**

STRENGTH DESIGN TABLE

A7+
Quick-Cure Adhesive

Rebar- ASTM A615 Grade 60 Steel in Uncracked Concrete - Tension (lbf) and Shear (lbf)

Rebar	Anchor Diameter (in.)	Embedment Depth (in.)	Tension (lbf)					Shear (lbf)
			2500 psi	3000 psi	4000 psi	5000 psi	6000 - 8000 psi	2500 - 8000 psi
#3	3/8	3 3/8	3,663	3,663	3,663	3,663	3,663	3,564
		4 1/2	4,884	4,884	4,884	4,884	4,884	3,564
		7 1/2	6,435	6,435	6,435	6,435	6,435	3,564
#4	1/2	4 1/2	7,446	7,523	7,523	7,523	7,523	6,480
		6	10,030	10,030	10,030	10,030	10,030	6,480
		10	11,700	11,700	11,700	11,700	11,700	6,480
#5	5/8	5 5/8	10,406	11,399	11,542	11,542	11,542	10,044
		7 1/2	15,389	15,389	15,389	15,389	15,389	10,044
		12 1/2	18,135	18,135	18,135	18,135	18,135	10,044
#6	3/4	6 3/4	13,679	14,871	14,871	14,871	14,871	14,256
		9	19,827	19,827	19,827	19,827	19,827	14,256
		15	25,740	25,740	25,740	25,740	25,740	14,256
#7	7/8	7 7/8	17,237	18,883	19,467	19,467	19,467	19,440
		10 1/2	25,955	25,955	25,955	25,955	25,955	19,440
		17 1/2	35,100	35,100	35,100	35,100	35,100	19,440
#8	1	9	21,060	23,070	25,115	25,115	25,115	25,596
		12	32,424	33,486	33,486	33,486	33,486	25,596
		20	46,215	46,215	46,215	46,215	46,215	25,596
#9	1 1/8	10 3/16	25,363	27,638	31,472	31,472	31,472	32,400
		13 1/2	38,845	41,816	41,816	41,816	41,816	32,400
		22 9/16	58,500	58,500	58,500	58,500	58,500	32,400
#10	1 1/4	11 1/2	30,491	33,018	38,477	43,019	46,227	41,148
		15 1/4	46,406	50,835	58,699	61,261	61,261	41,148
		25 7/16	74,295	74,295	74,295	74,295	74,295	41,148

Tabulated values are for estimation puposes only and should not be used for design (please use our TruSpec anchorage design software at www.itwredhead.com)
Tabulated values represent design strengths per ACI 318 for a single anchor in adequate concrete thickness, not near an edge nor adjacent achnorage, not for sustained nor seismic loading
Bond strengths are for dry, cracked concrete with periodic inspection.
Bond strengths are for Temperature Range A (maximum long term temperature 110F, maximum short term temp 130F).

STRENGTH DESIGN TABLE

A7+ Quick-Cure Adhesive

Threaded Rod- ASTM A193 B7 in Uncracked Concrete

Anchor Diameter (in.)	Embedment Depth (in.)	Tension (lbf)					Shear (lbf)
		2500 psi	3000 psi	4000 psi	5000 pso	6000 psi - 8000 psi	2500 psi - 8000 psi
3/8	3 3/8	3,871	3,871	3,871	3,871	3,871	3,777
	4 1/2	5,161	5,161	5,161	5,161	5,161	3,777
	7 1/2	7,268	7,268	7,268	7,268	7,268	3,777
1/2	4 1/2	6,881	6,881	6,881	6,881	6,881	6,916
	6	9,175	9,175	9,175	9,175	9,175	6,916
	10	13,305	13,305	13,305	13,305	13,305	6,916
5/8	5 5/8	10,406	10,406	10,406	10,406	10,406	11,018
	7 1/2	14,336	14,336	14,336	14,336	14,336	11,018
	12 1/2	21,188	21,188	21,188	21,188	21,188	11,018
3/4	6 3/4	13,679	14,984	14,984	14,984	15,483	16,309
	9	20,644	20,644	20,644	20,644	20,644	16,309
	15	31,358	31,358	31,358	31,358	31,358	16,309
7/8	7 7/8	17,237	17,740	17,740	17,740	17,740	22,510
	10 1/2	23,654	23,654	23,654	23,654	23,654	22,510
	17 1/2	39,423	39,423	39,423	39,423	39,423	22,510
1	9	21,060	23,070	23,070	23,070	23,171	29,530
	12	30,894	30,894	30,894	30,894	30,894	29,530
	20	51,491	51,491	51,491	51,491	51,491	29,530
1 1/4	11 1/2	30,419	33,322	38,477	43,019	43,738	47,242
	15 1/4	46,406	50,835	57,962	57,962	57,962	47,242
	25 7/16	90,855	90,855	90,855	90,855	90,855	47,242

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Bond strengths are for dry, cracked concrete with periodic inspection.
Bond strengths are for Temperature Range A (maximum long term temperature 110F, maximum short term temp 130F).

STRENGTH DESIGN TABLE

A7+ Quick-Cure Adhesive

Threaded Rod in 2,500 - 8,000 psi Uncracked Concrete - Tension (lbf) and Shear (lbf)

Anchor Diameter (in.)	Embedment Depth (in.)	Carbon Steel A36		Stainless Steel F593		ASTM A193 B7 Threaded Rod	
		Tension (lbf)	Shear (lbf)	Tension (lbf)	Shear (lbf)	Tension (lbf)	Shear (lbf)
3/8	3 3/8	3,375	1,755	3,871	2,280	3,871	3,777
	4 1/2	3,375	1,755	4,787	2,280	5,161	3,777
	7 1/2	3,375	1,755	4,787	2,280	7,268	3,777
1/2	4 1/2	6,173	3,211	6,881	4,044	6,881	6,916
	6	6,173	3,211	8,762	4,044	9,175	6,916
	10	6,173	3,211	8,762	4,044	13,305	6,916
5/8	5 5/8	9,833	5,116	10,752	6,441	10,752	11,018
	7 1/2	9,833	5,116	13,956	6,441	14,336	11,018
	12 1/2	9,833	5,116	13,956	6,441	21,188	11,018
3/4	6 3/4	14,550	7,566	15,483	7,614	15,483	16,309
	9	14,550	7,566	16,500	7,614	20,644	16,309
	15	14,550	7,566	16,500	7,614	31,358	16,309
7/8	7 7/8	17,740	10,446	17,740	10,533	17,740	22,510
	10 1/2	20,085	10,446	22,822	10,533	23,654	22,510
	17 1/2	20,085	10,446	22,822	10,533	39,423	22,510
1	9	23,171	13,702	23,171	13,818	23,171	29,530
	12	26,348	13,702	29,936	13,818	30,894	29,530
	20	26,348	13,702	29,936	13,818	51,491	29,530
1 1/4	11 1/2	38,477	21,925	38,477	22,092	38.477	47,242
	15 1/4	42,158	21,925	47,869	22,092	57,049	47,242
	25 7/16	42,158	21,925	47,869	22,092	90,855	47,242

Tabulated values are for estimation puposes only and should not be used for design (please use our TruSpec anchorage design software at www.itwredhead.com)
Tabulated values represent design strengths per ACI 318 for a single anchor in adequate concrete thickness, not near an edge nor adjacent achnorage, not for sustained nor seismic loading
Bond strengths are for dry, cracked concrete with periodic inspection.
Bond strengths are for Temperature Range A (maximum long term temperature 110F, maximum short term temp 130F).

STRENGTH DESIGN TABLE

A7+
Quick-Cure Adhesive

Rebar- ASTM A615 Grade 60 Steel in Cracked Concrete - Tension (lbf) and Shear (lbf)

Rebar	Anchor Diameter (in.)	Embedment Depth (in.)	Tension (lbf) 2500 - 8000 psi concrete	Shear (lbf) 2500 - 8000 psi concrete
#3	3/8	3 3/8	1,651	2,311
		4 1/2	2,201	3,082
		7 1/2	3,669	3,564
#4	1/2	4 1/2	2,935	4,109
		6	3,914	5,479
		10	6,523	6,480
#5	5/8	5 5/8	4,586	6,421
		7 1/2	6,115	8,561
		12 1/2	10,192	10,044
#6	3/4	6 3/4	5,117	7,164
		9	6,823	9,552
		15	11,372	14,256
#7	7/8	7 7/8	6,965	9,751
		10 1/2	9,287	13,002
		17 1/2	15,478	19,440
#8	1	9	9,097	12,736
		12	12,130	16,982
		20	20,216	25,596
#9	1 1/8	10 3/16	11,616	16,262
		13 1/2	15,434	21,607
		22 9/16	25,726	32,400
#10	1 1/4	11 1/2	17,447	24,426
		15 1/4	23,121	32,369
		25 7/16	38,592	41,148

Tabulated values are for estimation puposes only and should not be used for design (please use our TruSpec anchorage design software at www.itwredhead.com)
Tabulated values represent design strengths per ACI 318 for a single anchor in adequate concrete thickness, not near an edge nor adjacent achnorage, not for sustained nor seismic loading
Bond strengths are for dry, cracked concrete with periodic inspection.
Bond strengths are for Temperature Range A (maximum long term temperature 110F, maximum short term temp 130F).

STRENGTH DESIGN TABLE

A7+ Quick-Cure Adhesive

Threaded Rod in 2,500 - 8,000 psi Cracked Concrete - Tension (lbf) and Shear (lbf)

Anchor Diameter (in.)	Embedment Depth (in.)	Tension (lbf)	Shear (lbf)		
			Carbon Steel A36	Stainless Steel F593	ASTM A193 B7 Threaded Rod
3/8	3 3/8	2,318	1,755	2,280	3,245
	4 1/2	3,091	1,755	2,280	3,777
	7 1/2	5,151	1,755	2,280	3,777
1/2	4 1/2	3,071	3,211	4,044	4,300
	6	4,095	3,211	4,044	5,733
	10	6,825	3,211	4,044	6,916
5/8	5 5/8	5,224	5,116	6,441	7,314
	7 1/2	6,965	5,116	6,441	9,752
	12 1/2	11,609	5,116	6,441	11,018
3/4	6 3/4	7,785	7,566	7,614	10,899
	9	10,380	7,566	7,614	14,532
	15	17,300	7,566	7,614	16,309
7/8	7 7/8	8,275	10,446	10,533	11,585
	10 1/2	11,033	10,446	10,533	15,446
	17 1/2	18,388	10,446	10,533	22,510
1	9	10,186	13,702	13,818	14,260
	12	13,581	13,702	13,818	19,014
	20	22,635	13,702	13,818	29,530
1 1/4	11 1/2	17,172	21,925	22,092	24,041
	15 1/4	22,757	21,925	22,092	31,860
	25 7/16	37,984	21,925	22,092	47,242

Tabulated values are for estimation puposes only and should not be used for design (please use our TruSpec anchorage design software at www.itwredhead.com)
Tabulated values represent design strengths per ACI 318 for a single anchor in adequate concrete thickness, not near an edge nor adjacent achnorage, not for sustained nor seismic loading
Bond strengths are for dry, cracked concrete with periodic inspection.
Bond strengths are for Temperature Range A (maximum long term temperature 110F, maximum short term temp 130F).

MASONRY DESIGN TABLE

A7+ Quick-Cure Adhesive

Grout-filled Concrete Block: Allowable Tension and Shear Loads based on Steel Design Information for U.S. Customary Unit Threaded Rod [1, 2, 3]

Anchor Diameter (in.)	Tension (lb)			Shear (lb)		
	ASTM A307 F_u = 60 ksi	ASTM A193 Grade B7 F_u = 125 ksi	ASTM F593 SS 304 F_u = 100 ksi	ASTM A307 F_u = 60 ksi	ASTM A193 Grade B7 F_u = 125 ksi	ASTM F593 SS 304 F_u = 100 ksi
3/8	2,185	4,555	3,645	1,125	2,345	1,875
1/2	3,885	8,100	6,480	2,000	4,170	3,335
5/8	6,075	12,655	10,125	3,130	6,520	5,215
3/4	8,750	18,225	12,390	4,505	9,390	6,385

For SI: 1 inch = 25.4mm, 1 lbf = 4.45N, 1ft-lbf = 1.356 N-M, 1 psi = 0.006895 MPa

[1]Allowable load used in the design must be the lesser of bond values and tabulated steel element values.

[2]Allowable tension and shear loads for threaded rods to resist short term loads, such as wind or seismic, must be calculated in accordance with Section 4.1 as applicable.

[3]Allowable steel loads are based on allowable tension and shear stresses equal to 0.33X Fu and 0.17xFu, respectively.

MASONRY DESIGN TABLE

A7+ Quick-Cure Adhesive

Grout-filled Concrete Block: Allowable Tension Loads for Threaded Rod [1, 2, 3, 4, 7, 9, 10, 11, 12]

Anchor Diameter (in.)	Minimum Embedment (inches)	Load at s_{cr} and c_{cr} (lb)	Spacing[5]			Edge Distance[6]		
			Critical s_{cr} (inches)	Minimum s_{min} (inches)	Load reduction factor for s_{min} [8]	Critical c_{cr} (inches)	Minimum c_{min} (inches)	Load reduction factor for c_{min} [8]
3/8	3 3/8	1,125	13.5	4	1.00	12	4	1.00
1/2	4 1/2	1,695	18	4	0.60	20	4	0.90
5/8	5 5/8	2,015	22.5	4	0.60	20	4	0.90
3/4	6 3/4	3,145	27	4	0.60	20	4	0.63

MASONRY DESIGN TABLE

A7+ Quick-Cure Adhesive

Grout-filled Concrete Block: Allowable Shear Loads for Threaded Rod [1, 2, 3, 4, 7, 9, 10, 11, 12]

Anchor Diameter (in.)	Minimum Embedment (inches)	Load at s_{cr} and c_{cr} (lb)	Spacing[5]			Edge Distance[6]		
			Critical s_{cr} (inches)	Minimum s_{min} (inches)	Load reduction factor for s_{min} [8]	Critical c_{cr} (inches)	Minimum c_{min} (inches)	Load reduction factor for c_{min} [8]
3/8	3 3/8	750	13.5	4	0.50	12	4	0.95
1/2	4 1/2	1,520	18	4	0.50	20	4	0.44
5/8	5 5/8	2,285	22.5	4	0.50	12	4	0.26
3/4	6 3/4	2,345	27	4	0.50	20	4	0.26

For SI: 1 inch = 25.4mm, 1 lbf = 0.0044 kN, 1 ksi = 6.894 MPa. (Refer to Table 4 for footnotes)

[1]All values are for anchors installed in fully grouted concrete masonry with minimum masonry strength of 1500 psi (10.3 MPa). Concrete masonry units must be light-, medium, or normal-weight conforming to ASTM C 90. Allowable loads have been calculated using a safety factor of 5.0.

[3]Anchors may be installed in any location in the face of the masonry wall (cell, web, bed joint) as shown in Figure 2.

[4]A maximum of two anchors may be installed in a single masonry cell in accordance with the spacing and edge or end distance requirements. Embedment is measured from the outside surface of the concrete masonry unit to the embedded end of the anchor. See Figure 2 of this report.

[5]The critical spacing distance, scr, is the anchor spacing where full load values in the table may be used. The minimum spacing distance, smin, is the minimum anchor spacing for which values are available and installation is permitted. Spacing distance is measured from the centerline to centerline between two anchors.

[6]The critical edge or end distance, ccr, is the distance where full load values in the table may be used. The minimum edge or end distance, cmin, is the minimum distance for which values are available and installation is permitted. Edge or end distance is measured from anchor centerline to the closest unrestrained edge.

[7]The tabulated values are applicable for anchors in the ends of grout-filled concrete masonry units where minimum edge distances are maintained.

[8]Load values for anchors installed less than scr and ccr must be multiplied by the appropriate load reduction factor based on actual spacing (s) or edge distance (c). Load factors are multiplicative; both spacing and edge reduction factors must be considered.

[9]Linear interpolation of load values between minimum spacing (smin) and critical spacing (scr) and between minimum edge or end distance (cmin) and critical edge or end distance (ccr) is permitted.

[10]Concrete masonry width (wall thickness) must be equal to or greater than 1.5 times the anchor embedment depth (e.g. 3/8-inch- and 1/2-inch-diameter anchors are permitted in minimum nominally 6-inch-thick concrete masonry). The 5/8- and 3/4-inch-diameter anchors must be installed in minimum nominally 8-inch-thick concrete masonry.

[11]Allowable loads must be the lesser of the adjusted masonry or bond values tabulated above and the steel strength values given in Table 2.

[12]Tabulated allowable bond loads must be adjusted for increased in-service base material temperatures in accordance with Figure 1, as applicable.

MASONRY DESIGN TABLE

A7+ Quick-Cure Adhesive

Grout-filled Concrete Block: Allowable Tension and Shear Loads for Rebar [1, 2, 3]

Rebar Size	Tension (lb)	Shear (lb)
	ASTM A615, Grade 60	ASTM A615, Grade 60
No. 3	3,270	1,685
No. 4	5,940	3,060
No. 5	9,205	4,745
No. 6	13,070	6,730

For SI: 1 inch = 25.4mm, 1 lbf = 4.45N, 1ft-lbf = 1.356 N-M, 1 psi = 0.006895 MPa

1Allowable load used in the design must be the lesser of bond values and tabulated steel element values.

2Allowable tension and shear loads for threaded rods to resist short term loads, such as wind or seismic, must be calculated in accordance with Section 4.1 as applicable.

3Allowable steel loads are based on allowable tension and shear stresses equal to 0.33X Fu and 0.17xFu, respectively.

MASONRY DESIGN TABLE

A7+ Quick-Cure Adhesive

Grout-filled Concrete Block: Allowable Tension Loads for Rebar [1, 2, 3, 4, 7, 9, 10, 11, 12]

Anchor Diameter (in.)	Minimum Embedment (inches)	Load at s_{cr} and c_{cr} (lb)	Spacing[5]			Edge Distance[6]		
			Critical s_{cr} (inches)	Minimum s_{min} (inches)	Load reduction factor for s_{min} [8]	Critical c_{cr} (inches)	Minimum c_{min} (inches)	Load reduction factor for c_{min} [8]
3/8	3 3/8	1,530	13.5	4	1.00	12	4	1.00
1/2	4 1/2	1,845	18	4	0.60	20	4	0.90
5/8	5 5/8	2,465	22.5	4	0.60	20	4	0.90
3/4	6 3/4	2,380	27	4	0.60	20	4	0.63

MASONRY DESIGN TABLE

A7+ Quick-Cure Adhesive

Grout-filled Concrete Block: Allowable Shear Loads for Rebar [1, 2, 3, 4, 7, 9, 10, 11, 12]

Anchor Diameter (in.)	Minimum Embedment (inches)	Load at s_{cr} and c_{cr} $\perp$ to edge (lb)	Spacing[5]			Edge Distance[6]		
			Critical s_{cr} (inches)	Minimum s_{min} (inches)	Load reduction factor for s_{min} [8]	Critical c_{cr} (inches)	Minimum c_{min} (inches)	Load reduction factor for c_{min} [8]
3/8	3 3/8	1,410	13.5	4	0.50	12	4	0.95
1/2	4 1/2	1,680	18	4	0.50	20	4	0.44
5/8	5 5/8	3,245	22.5	4	0.50	12	4	0.26
3/4	6 3/4	4,000	27	4	0.50	20	4	0.26

For SI: 1 inch = 25.4 mm; 1 lbf = 0.0044 kN, 1 ksi = 6.894 MPa.

(The following footnotes apply to both Tables 6 and 7)

1All values are for anchors installed in fully grouted concrete masonry with minimum masonry strength of 1500 psi (10.3 MPa). Concrete masonry units must be light-, medium, or normal-weight conforming to ASTM C 90. Allowable loads have been calculated using a safety factor of 5.0.

3Anchors may be installed in any location in the face of the masonry wall (cell, web, bed joint) as shown in figure 2.

4A maximum of two anchors may be installed in a single masonry cell in accordance with the spacing and edge or end distance requirements. Embedment is measured from the outside surface of the concrete masonry unit to the embedded end of the anchor. See Figure 2 of this report.

5The critical spacing distance, scr, is the anchor spacing where full load values in the table may be used. The minimum spacing distance, smin, is the minimum anchor spacing for which values are available and installation is permitted. Spacing distance is measured from the centerline to centerline between two anchors.

6The critical edge or end distance, ccr, is the distance where full load values in the table may be used. The minimum edge or end distance, cmin, is the minimum distance for which values are available and installation is permitted. Edge or end distance is measured from anchor centerline to the closest unrestrained edge.

7The tabulated values are applicable for anchors in the ends of grout-filled concrete masonry units where minimum edge distances are maintained.

8Load values for anchors installed less than scr and ccr must be multiplied by the appropriate load reduction factor based on actual spacing (s) or edge distance (c). Load factors are multiplicative; both spacing and edge reduction factors must be considered.

9Linear interpolation of load values between minimum spacing (smin) and critical spacing (scr) and between minimum edge or end distance (cmin) and critical edge or end distance (ccr) is permitted.

10Concrete masonry width (wall thickness) must be equal to or greater than 1.5 times the anchor embedment depth (e.g. No. 3 and No. 4 reinforcing bars are permitted in minimum nominally 6-inch-thick concrete masonry). No. 5 and No. 6 reinforcing bars must be installed in minimum nominally 8-inch-thick concrete masonry.

11Allowable loads must be the lesser of the adjusted masonry or bond values tabulated above and the steel strength values given in Table 4.

12Tabulated allowable bond loads must be adjusted for increased in-service base material temperatures in accordance with Figure 1, as applicable.

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SECTION: 03 16 00—CONCRETE ANCHORS
DIVISION: DIVISION: 05 00 00—METALS
SECTION: 05 05 19—POST-INSTALLED CONCRETE ANCHORS

REPORT HOLDER:

ITW RED HEAD

700 HIGH GROVE BOULEVARD
GLENDALE HEIGHTS, ILLINOIS 60139

EVALUATION SUBJECT:

ITW RED HEAD A7+ ADHESIVE ANCHORING SYSTEM FOR CRACKED AND UNCRACKED CONCRETE

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DIVISION: 03 00 00—CONCRETE
Section: 03 16 00—Concrete Anchors

DIVISION: 05 00 00—METALS
Section: 05 05 19—Post-Installed Concrete Anchors

REPORT HOLDER:

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EVALUATION SUBJECT:

ITW RED HEAD A7+ ADHESIVE ANCHORING SYSTEM FOR CRACKED AND UNCRACKED CONCRETE

1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2015, 2012, 2009, 2006 and 2003 *International Building Code*® (IBC)
- 2015, 2012, 2009, 2006 and 2003 *International Residential Code*® (IRC)
- 2013 *Abu Dhabi International Building Code* (ADIBC)†

†The ADIBC is based on the 2009 IBC. 2009 IBC code sections referenced in this report are the same sections in the ADIBC.

Property evaluated:

Structural

2.0 USES

The Red Head A7+ Adhesive Anchoring System is a post-installed anchorage system used to resist static, wind or earthquake (Seismic Design Categories A through F) tension and shear loads in cracked and uncracked normal-weight concrete having a specified compressive strength, f'_c, of 2500 psi to 8,500 psi (17.2 MPa to 58.6 MPa) [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1].

The anchoring system complies with anchors as described in Section 1901.3 of the 2015 IBC, Section 1909 of the 2012 IBC and is an alternative to cast-in-place anchors described in Section 1908 of the 2012 IBC, and Sections 1911 and 1912 of the 2009 and 2006 IBC, and Sections 1912 and 1913 of the 2003 IBC. The anchoring system may also be used where an engineered design is submitted in accordance with Section R301.1.3 of the IRC.

3.0 DESCRIPTION

3.1 General:

The Red Head Epcon A7+ Adhesive Anchoring System is a two-component, high-strength, fast-cure, structural adhesive, used with continuously threaded rods and deformed reinforcing bar installed in normal-weight concrete. The primary components of the ITW Red Head A7+ Adhesive Anchoring System are shown in Figure 1 of this report.

The manufacturer's printed installation instructions (MPII) are included with the adhesive packaging and are replicated in Figure 3 of this report.

3.2 Materials:

3.2.1 Adhesive: The Red Head A7+ Anchoring System is a two-part vinylester packaged in a dual-chamber cartridge at a volumetric ratio of 10:1. The cartridge is available in 28-ounce (side-by-side), 9.5-ounce (coaxial), and 5-ounce (coaxial) sizes. The components are dispensed through a static mixing nozzle which attaches to the cartridge. The original, unopened cartridge has a shelf life of 18 months, as indicated by the "best used by" date stamped onto the cartridge, when stored in a cool, dry, ventilated area and in accordance with Figure 3.

3.2.2 Hole Cleaning Equipment: Hole cleaning equipment consists of wire brushes, as shown in Figures 1 and 3, and a compressed air nozzle with extension.

3.2.3 Dispensing Tools: Red Head A7+ Adhesive must be dispensed with manual or pneumatic dispensing tools provided by ITW Red Head, as shown in Figure 1.

3.2.4 Anchor Elements:

3.2.4.1 Threaded Rods: The continuously threaded rods range from $^3/_8$ inch through $1^1/_4$ inches (9.5 mm through 31.75 mm) in diameter. Carbon steel threaded rods must comply with either ASTM A36 [minimum f_{uta} = 58,000 psi (400 MPa)] or ASTM A193, Grade B7 [minimum f_{uta} = 125,000 psi (860 MPa)]. Stainless steel threaded rods must comply with ASTM F593 (Alloy Type 300, CW1 and CW2) [minimum f_{uta} = 95,000 psi (655 MPa) for CW1, and f_{uta} =80,000 psi (552 MPa) for CW2]. Table 1 notes steel design information for the threaded rods. Carbon steel threaded rods must be furnished with a minimum 0.0002-inch-thick (5 µm) zinc electroplated coating complying with ASTM B633 SC1 or must be hot-dipped galvanized complying with ASTM A153, Class C or D.

Threaded steel rods must be straight and free from indentations or other defects along their length.

3.2.4.2 Steel Reinforcing Bars: Steel reinforcing bars are deformed reinforcing bars as described in Table 4 of this report. The embedded portions of reinforcing bars must be straight, and free of mill scale, rust, mud, oil, and other coatings that may impair the bond with the adhesive. Reinforcing bars must not be bent after installation, except as set forth in ACI 318-14 Section 26.6.3.1 (b) or ACI 318-11 Section 7.3.2, as applicable, with the additional condition that the bars must be bent cold, and heating of reinforcing bars to facilitate field bending is not permitted.

3.2.4.3 Ductility: In accordance with ACI 318-14 2.3 or ACI 318-11 D.1, as applicable, in order for a steel element to be considered ductile, the tested elongation must be at least 14 percent and reduction of area must be at least 30 percent. Steel elements with a tested elongation of less than 14 percent or a reduction of area of less than 30 percent, or both, are considered brittle. Where values are nonconforming or unstated, the steel must be considered brittle.

3.3 Concrete:

Normal-weight concrete must comply with Sections 1903 and 1905 of the IBC. The specified compressive strength of the concrete must be from 2,500 psi to 8,500 psi (17.2 MPa to 58.6 MPa) [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1].

4.0 DESIGN AND INSTALLATION

4.1 Strength Design:

4.1.1 General: The design strength of anchors under the 2015 IBC, as well as the 2015 IRC must be determined in accordance with ACI 318-14 and this report. The design strength of anchors under the 2012, 2009, 2006 and 2003 IBC, as well as the 2012, 2009, 2006 and 2003 IRC, must be determined in accordance with ACI 318-11 and this report.

A design example in accordance with the 2012 IBC based on ACI 318-11 is provided in Figure 2 of this report.

Design parameters are based on ACI 318-14 for use with the 2015 IBC, and the ACI 318-11 for use with the 2012, 2009, 2006 and 2003 IBC unless noted otherwise in this report.

The strength design of anchors must comply with ACI 318-14 17.3.1 or ACI 318-11 D.4.1, as applicable, except as required in ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable. Design parameters are provided in Tables 1 through 6. Strength reduction factors, ϕ, as given in ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, must be used for load combinations calculated in accordance with Section 1605.2 of the IBC or ACI 318-14 5.3 or ACI 318-11 9.2, as applicable. Strength reduction factors, ϕ, as described in ACI 318-11 D.4.4, must be used for load combinations calculated in accordance with ACI 318-11 Appendix C.

4.1.2 Static Steel Strength in Tension: The nominal static steel strength of a single anchor in tension, N_{sa}, in accordance with ACI 318-14 17.4.1.2 or ACI 318-11 D.5.1.2, as applicable, and the associated strength reduction factors, ϕ, in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, are given in Tables 1 and 4 of this report for the anchor element types included in this report.

4.1.3 Static Concrete Breakout Strength in Tension: The nominal static concrete breakout strength of a single anchor or group of anchors in tension, N_{cb} or N_{cbg}, must be calculated in accordance with ACI 318-14 17.4.2 or ACI 318-11 D.5.2, as applicable, with the following addition:

The basic concrete breakout strength of a single anchor in tension, N_b, must be calculated in accordance with ACI 318-14 17.4.2.2 or ACI 318-11 D.5.2.2, as applicable, using the values of $k_{c,cr}$, and $k_{c,uncr}$ as described in Tables 2 and 5 of this report. Where analysis indicates no cracking in accordance with ACI 318-14 17.4.2.6 or ACI 318-11 D.5.2.6, as applicable, N_b must be calculated using $k_{c,uncr}$ and $\Psi_{c,N}$ = 1.0. For anchors in lightweight concrete see ACI 318-14 17.2.6 or ACI 318-11 D.3.6, as applicable. The value of f'_c used for calculation must be limited to 8,000 psi (55 MPa) in accordance with ACI 318-14 17.2.7 or ACI 318-11 D.3.7, as applicable. Additional information for the determination of nominal bond strength in tension is given in Section 4.1.4 of this report.

4.1.4 Static Bond Strength in Tension: The nominal static bond strength of a single adhesive anchor or group of adhesive anchors in tension, N_a or N_{ag}, must be calculated in accordance with ACI 318-14 17.4.5 or ACI 318-11 D.5.5, as applicable. Bond strength values are a function of whether the concrete is cracked or uncracked, the concrete temperature range, the installation conditions (dry or water-saturated concrete, water-filled holes, or submerged), and the level of inspection provided. The resulting characteristic bond strength shall be multiplied by the associated strength reduction factor ϕ_{nn} as follows:

CONCRETE TYPE	PERMISSIBLE INSTALLATION CONDITIONS	BOND STRENGTH	ASSOCIATED STRENGTH REDUCTION FACTOR
Uncracked	Dry	τ_{uncr}	ϕ_d
	Water-saturated	τ_{uncr}	ϕ_{ws}
	Water-filled holes	τ_{uncr}	ϕ_{wf}
	Submerged	τ_{uncr}	ϕ_{sub}
Cracked	Dry	τ_{cr}	ϕ_d
	Water-saturated	τ_{cr}	ϕ_{ws}
	Water-filled holes	τ_{cr}	ϕ_{wf}
	Submerged	τ_{cr}	ϕ_{sub}

Strength reduction factors for determination of the bond strength are given in Tables 3 and 6 of this report.

4.1.5 Static Steel Strength in Shear: The nominal static strength of a single anchor in shear as governed by the steel, V_{sa}, in accordance with ACI 318-14 17.5.1.2 or ACI 318-11 D.6.1.2, as applicable, and strength reduction factors, ϕ, in accordance with ACI 318-14 17.2.3 or ACI 318-11 D.4.3, as applicable are given in Tables 1 and 4 of this report for the anchor element types included in this report.

4.1.6 Static Concrete Breakout Strength in Shear: The nominal concrete breakout strength of a single anchor or group of anchors in shear, V_{cb} or V_{cbg}, must be calculated in accordance with ACI 318-14 17.5.2 or ACI 318-11 D.6.2, as applicable, based on information given in Table 2 and Table 5 of this report. The basic concrete breakout strength of a single anchor in shear, V_b, must be calculated in accordance with ACI 318-14 17.5.2.2 or ACI 318-11 D.6.2.2, as applicable, using the values of d given in this report in lieu of d_a (2015, 2012 and 2009 IBC), d_o (2006 IBC). In addition, h_{ef} shall be substituted for ℓ_e. In no case shall ℓ_e exceed 8d. The value of f'_c must be limited to a

maximum value of 8,000 psi (55 MPa) in accordance with ACI 318-14 17.2.7 or ACI 318-11 D.3.7, as applicable.

4.1.7 Static Concrete Pryout Strength in Shear: The nominal static pryout strength of a single anchor or group of anchors in shear, V_{cp} or V_{cpg}, shall be calculated in accordance with ACI 318-14 17.5.3 or ACI 318-11 D.6.3, as applicable.

4.1.8 Interaction of Tensile and Shear Forces: For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-14 17.6 or ACI 318-11 D.7, as applicable.

4.1.9 Minimum Member Thickness, h_{min}, Anchor Spacing, s_{min}, and Edge Distance, c_{min}: In lieu of ACI 318-14 17.7.1 and 17.7.3 or ACI 318-11 D.8.1 and D.8.3, as applicable, values of s_{min} and c_{min}, as given in Table 2 and Table 5 of this report, must be observed for anchor design and installation. The minimum member thicknesses h_{min}, as given in Table 2 and Table 5 of this report must be observed for anchor design and installation. For adhesive anchors that will remain untorqued, ACI 318-14 17.7.4 or ACI 318-11 D.8.4, as applicable, applies.

4.1.10 Critical Edge Distance c_{ac} and $\psi_{cp,Na}$: The modification factor $\psi_{cp,Na}$, must be determined in accordance with ACI 318-14 17.4.5.5 or ACI 318-11 D.5.5.5, as applicable, except as noted below:

For all cases where $c_{Na}/c_{ac}<1.0$, $\psi_{cp,Na}$ determined from ACI 318-14 Eq. 17.4.5.5b or ACI 318-11 Eq. D-27, as applicable, need not be taken less than c_{Na}/c_{ac}. For all other cases, $\psi_{cp,Na}$ shall be taken as 1.0.

The critical edge distance, c_{ac} must be calculated according to Eq. 17.4.5.5c for ACI 318-14 or Eq. D-27a for ACI 318-11, in lieu of ACI 318-14 17.7.6 or ACI 318-11 D.8.6, as applicable.

$$c_{ac}=h_{ef}\cdot\left(\frac{\tau_{k,\,uncr}}{1160}\right)^{0.4}\cdot\left[3.1-0.7\frac{h}{h_{ef}}\right]$$

(Eq. 17.4.5.5c for ACI 318-14 or Eq. D-27a for ACI 318-11)

where

$\left[\frac{h}{h_{ef}}\right]$ need not be taken as larger than 2.4; and

$\tau_{k,uncr}$ = the characteristic bond strength stated in the tables of this report whereby $\tau_{k,uncr}$ need not be taken as larger than:

$$\tau_{uncr}=\frac{k_{uncr}\sqrt{h_{ef}f_c'}}{\pi\cdot d_a} \qquad \text{Eq. (4-1)}$$

4.1.11 Design Strength in Seismic Design Categories C, D, E and F: In structures assigned to Seismic Design Category C, D, E or F under the IBC or IRC, anchors must be designed in accordance with ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable, except as described below. Modifications to ACI 318-14 17.2.3 shall be applied under Section 1905.1.8 of the 2015 IBC. For the 2012 IBC, Section 1905.1.9 shall be omitted. The nominal steel shear strength, V_{sa}, must be adjusted by $\alpha_{V,seis}$ as given in Tables 1 and 4 for the anchor element types included in this report. The nominal bond strength $\tau_{\kappa,cr}$ must be adjusted by $\alpha_{N,seis}$, as given in Tables 3 and 6 of this report.

As an exception to ACI 318-11 D.3.3.4.2: Anchors designed to resist wall out-of-plane forces with design strengths equal to or greater than the force determined in accordance with ASCE 7 Equation 12.11-1 or 12.14-10 shall be deemed to satisfy ACI 318-11 D.3.3.4.3(d).

Under ACI 318-11 D.3.3.4.3(d), in lieu of requiring the anchor design tensile strength to satisfy the tensile strength requirements of ACI 318-11 D.4.1.1, the anchor design tensile strength shall be calculated from ACI 318-11 D.3.3.4.4.

The following exceptions apply to ACI 318-11 D.3.3.5.2:

1. For the calculation of the in-plane shear strength of anchor bolts attaching wood sill plates of bearing or non-bearing walls of light-frame wood structures to foundations or foundation stem walls, the in-plane shear strength in accordance with ACI 318-11 D.6.2 and D.6.3 need not be computed and ACI 318-11 D.3.3.5.3 need not apply provided all of the following are satisfied:

1.1. The allowable in-plane shear strength of the anchor is determined in accordance with AF&PA NDS Table 11E for lateral design values parallel to grain.

1.2. The maximum anchor nominal diameter is $^5/_8$ inch (16 mm).

1.3. Anchor bolts are embedded into concrete a minimum of 7 inches (178 mm).

1.4. Anchor bolts are located a minimum of $1^3/_4$ inches (45 mm) from the edge of the concrete parallel to the length of the wood sill plate.

1.5. Anchor bolts are located a minimum of 15 anchor diameters from the edge of the concrete perpendicular to the length of the wood sill plate.

1.6. The sill plate is 2-inch or 3-inch nominal thickness.

2. For the calculation of the in-plane shear strength of anchor bolts attaching cold-formed steel track of bearing or non-bearing walls of light-frame construction to foundations or foundation stem walls, the in-plane shear strength in accordance with ACI 318-11 D.6.2 and D.6.3 need not be computed and ACI 318-11 D.3.3.5.3 need not apply provided all of the following are satisfied:

2.1. The maximum anchor nominal diameter is $^5/_8$ inch (16 mm).

2.2. Anchors are embedded into concrete a minimum of 7 inches (178 mm).

2.3. Anchors are located a minimum of $1^3/_4$ inches

(45 mm) from the edge of the concrete parallel to the length of the track.

2.4. Anchors are located a minimum of 15 anchor diameters from the edge of the concrete perpendicular to the length of the track.

2.5. The track is 33 to 68 mil designation thickness.

Allowable in-plane shear strength of exempt anchors, parallel to the edge of concrete shall be permitted to be determined in accordance with AISI S100 Section E3.3.1.

3. In light-frame construction, bearing or nonbearing walls, shear strength of concrete anchors less than or equal to 1 inch [25 mm] in diameter attaching a sill plate or track to foundation or foundation stem wall need not satisfy ACI 318-11 D.3.3.5.3(a) through (c) when the design strength of the anchors is determined in accordance with ACI 318-11 D.6.2.1(c).

4.2 Allowable Stress Design:

4.2.1 General: For anchors designed using load combinations in accordance with IBC Section 1605.3 (Allowable Stress Design), allowable loads shall be established using Eq. (4-2) or Eq. (4-3):

$$T_{allowable,ASD} = \frac{\phi N_n}{\alpha} \quad \text{Eq. (4-2)}$$

and

$$V_{allowable,ASD} = \frac{\phi V_n}{\alpha} \quad \text{Eq. (4-3)}$$

where

$T_{allowable,ASD}$ = Allowable tension load (lbf or kN)

$V_{allowable,ASD}$ = Allowable shear load (lbf or kN)

ϕN_n = Lowest design strength of an anchor or anchor group in tension as determined in accordance with ACI 318-14 Chapter 17 and 2015 IBC Section 1905.1.8, ACI 318-11 Appendix D, ACI 318-08 Appendix D and 2009 IBC Sections 1908.1.9 and 1908.1.10, ACI 318-05 Appendix D and 2006 IBC Section 1908.1.16, and Section 4.1 of this report, as applicable.

ϕV_n = Lowest design strength of an anchor or anchor group in shear as determined in accordance with ACI 318-14 Chapter 17 and 2015 IBC Section 1905.1.8, ACI 318-11 Appendix D, ACI 318-08 Appendix D and 2009 IBC Sections 1908.1.9 and 1908.1.10, ACI 318-05 Appendix D and 2006 IBC Section 1908.1.16, and Section 4.1 of this report, as applicable.

α = Conversion factor calculated as a weighted average of the load factors for the controlling load combination. In addition, α must include all applicable factors to account for non-ductile failure modes and required over-strength.

Limits on edge distance, anchor spacing and member thickness described in this report must apply.

Example calculations for derivation of $T_{allowable,ASD}$ are provided in Figure 2 and Table 7.

4.2.2 Interaction of tensile and shear forces: In lieu of ACI 318-14 17.6.1, 17.6.2 and 17.6.3 or ACI 318-11 D.7.1, D.7.2 and D.7.3, as applicable, interaction must be calculated as follows:

For shear loads $V \leq 0.2V_{allowable,ASD}$, the full allowable load in tension shall be permitted.

For tension loads $T \leq 0.2T_{allowable,ASD}$, the full allowable load in shear shall be permitted.

For all other cases:

$$\frac{T}{T_{allowable,ASD}} + \frac{V}{V_{allowable,ASD}} \leq 1.2 \quad \text{Eq. (4-4)}$$

4.3 Installation:

Installation parameters are illustrated in Figure 3 of this report. Installation must be in accordance with ACI 318-14 17.8.1 and 17.8.2 or ACI 318-11 D.9.1 and D.9.2, as applicable. Anchor locations must comply with this report and the plans and specifications approved by the code official. Installation of the Red Head A7+ Adhesive Anchor System must conform to the manufacturer's printed installation instructions included in each unit package, as provided in Figure 3 of this report.

The adhesive anchoring system may be used for floor (vertically down), wall (horizontal) and overhead applications. Horizontal and overhead applications are to be used with the $^3/_8$-inch (9.5 mm) through $1^1/_4$-inch-diameter (31 mm) threaded rods and reinforcing bars. The adhesive may be injected directly to the end of the hole using extension tubing (E916-6) for the ½-inch- through $1^1/_4$-inch-diameter anchors, and extension tubing (E25-6) for the $^3/_8$-inch-diameter anchors. Alternatively, the $^5/_8$-inch-(16 mm) through $1^1/_4$-inch-diameter (31 mm) threaded rod and reinforcing bars may be installed with a Red Head piston plug.

A demonstration video of the vertical down and overhead installation in dry concrete may be viewed from the following links:

http://go.iccsafe.org/l/25182/2016-09-20/bv3v8h for vertical down installation.

http://go.iccsafe.org/l/25182/2016-09-20/bv3v79 for overhead installation with piston plugs.

4.4 Special Inspection:

4.4.1 General: Installations may be made under continuous special inspection or periodic special inspection, as determined by the registered design professional. Table 3 and Table 6 of this report provide strength reduction factors, ϕ, corresponding to the type of inspection provided.

Continuous special inspection of adhesive anchors installed in horizontal or upwardly inclined orientations to resist sustained tension loads shall be performed in accordance with ACI 318-14 17.8.2.4 or ACI 318-11 D.9.2.4, as applicable.

Under the IBC, additional requirements as set forth in Section 1705.1.1 and Table 1705.3 of the 2015 or 2012 IBC, and Sections 1705, 1706 or 1707 of the 2009, 2006, and 2003 IBC must be observed, where applicable.

4.4.2 Continuous Special Inspection: Installations made under continuous special inspection with an on-site proof loading program must be performed in accordance with Section 1705.1.1 and Table 1705.3 of the 2015 and 2012 IBC, Sections 1704.4 and 1704.15 of the 2009 IBC, or Section 1704.13 of the 2006 and 2003 IBC, whereby continuous special inspection is defined in Section 1702.1 of the IBC, and this report. The special inspector must be on the jobsite continuously during anchor installation to verify anchor type, adhesive expiration date, anchor dimensions, concrete type, concrete compressive strength, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete thickness, anchor embedment, tightening torque, and adherence to the manufacturers printed installation instructions.

The proof loading program must be established by the registered design professional. As a minimum, the following requirements must be addressed in the proof loading program:

1. Frequency of proof loading based on anchor type, diameter, and embedment.
2. Proof loads by anchor type, diameter, embedment, and location.
3. Acceptable displacements at proof load.
4. Remedial action in the event of a failure to achieve proof load, or excessive displacement.

Unless otherwise directed by the registered design professional, proof loads must be applied as confined tension tests. Proof load levels must not exceed the lesser of 67 percent of the load corresponding to the nominal bond strength as calculated from the characteristic bond stress for uncracked concrete modified for edge effects and concrete properties, or 80 percent of the minimum specified anchor element yield strength ($A_{se,N} \cdot f_{ya}$). The proof load shall be maintained at the required load level for a minimum of 10 seconds.

4.4.3 Periodic Special Inspection: Periodic special inspection must be performed where required in accordance with Section 1705.1.1 and Table 1705.3 of the 2015 and 2012 IBC, Section 1704.15 and Table 1704.4 of

the 2009 IBC, or Section 1704.13 of the 2006, and 2003 IBC, whereby periodic special inspection is defined in Section 1702.1 of the IBC and this report. The special inspector must be on the jobsite initially during anchor installation to verify anchor type, anchor dimensions, concrete type, concrete compressive strength, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete thickness, anchor embedment, tightening torque, and adherence to the manufacturer's printed installation instructions. The special inspector must verify the initial installations of each type and size of adhesive anchor by construction personnel on the site. Subsequent installations of the same anchor type and size by the same construction personnel are permitted to be performed in the absence of the special inspector. Any change in the anchor product being installed or the personnel performing the installation requires an initial inspection. For ongoing installations over an extended period, the special inspector must make regular inspections to confirm correct handling and installation of the product.

4.5 Compliance with NSF/ANSI Standard 61:

The A7+ Adhesive Anchor System complies with the requirements of NSF/ANSI Standard 61, as referenced in Section 605 of the 2009 and 2006 *International Plumbing Code*® (IPC), and is certified for use as an anchoring adhesive for installing threaded rods less than or equal to 1.3 inches (33 mm) in diameter in concrete for water treatment applications. An NSF/ANSI Standard 61 listing is provided by NSF International.

5.0 CONDITIONS OF USE

The Red Head A7+ Adhesive Anchoring System described in this report complies with or is a suitable alternative to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions:

5.1 The Red Head A7+ Adhesive must be installed in accordance with the manufacturer's printed installation instructions, as included with the adhesive packaging and reproduced in Figure 3 of this report.

5.2 The anchors must be installed in cracked and uncracked normal-weight concrete having a specified compressive strength of f'_c = 2,500 psi to 8,500 psi (17.2 MPa to 58.6 MPa) [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1].

5.3 The values of f'_c used for calculation purposes must not exceed 8,000 psi (55 MPa).

5.4 Anchors must be installed in concrete base materials in holes predrilled in accordance with the instructions provided in Figure 3 of this report, using a carbide-tipped masonry drill bit manufactured within the range of the maximum and minimum drill-tip dimensions of ANSI B212.15-1994.

5.5 Loads applied to the anchors must be adjusted in accordance with Section 1605.2 of the IBC for strength design and in accordance with Section 1605.3 of the IBC for allowable stress design.

5.6 Red Head A7+ Adhesive Anchors are recognized for use in resisting short- and long-term loads, including wind and earthquake loads, subject to the conditions of this report.

5.7 In structures assigned to Seismic Design Category C, D, E or F under the IBC or IRC, anchor strength must comply with the requirements of 2009 IBC Section 1908.1.9 or 2006 IBC Section 1908.1.16

5.8 Red Head A7+ adhesive anchors are permitted to be installed in concrete that is cracked or that may be expected to crack during the service life of the anchor, subject to the conditions of this report.

5.9 Strength design values must be established in accordance with Section 4.1 of this report.

5.10 Allowable stress design values must be established in accordance with Section 4.2 of this report.

5.11 Minimum anchor spacing and edge distance, as well as minimum member thickness, must comply with the values given in this report.

5.12 Prior to anchor installation, calculations and details demonstrating compliance with this report must be submitted to the code official. The calculations and details must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.

5.13 Anchors are not permitted to support fire-resistive construction. Where not otherwise prohibited by the code, anchors are permitted for installation in fire-resistive construction provided at least one of the following conditions is fulfilled:

- Anchors are used to resist wind or seismic forces only.
- Anchors that support gravity load–bearing structural elements are within a fire-resistive envelope or a fire-resistive membrane, are protected by approved fire-resistive materials, or have been evaluated for resistance to fire exposure in accordance with recognized standards.
- Anchors are used to support nonstructural elements.

5.14 Since an ICC-ES acceptance criteria for evaluating data to determine the performance of adhesive anchors subjected to fatigue or shock loading is unavailable at this time, the use of these anchors under such conditions is beyond the scope of this report.

5.15 Use of zinc-plated carbon steel threaded rods or steel reinforcing bars is limited to dry, interior locations.

5.16 Use of hot-dipped galvanized carbon steel rods and stainless steel rods is permitted for exterior exposure or damp environments.

5.17 Steel anchoring materials in contact with preservative-treated and fire-retardant-treated wood must be of zinc-coated carbon steel or stainless steel. The minimum coating weights for zinc-coated steel must comply with ASTM A153.

5.18 Special inspection must be provided in accordance with Section 4.4 of this report. Continuous special inspection for anchors installed in horizontal or upwardly inclined orientations to resist sustained tension loads must be provided in accordance with Section 4.4 of this report.

5.19 Installation of anchors in horizontal or upwardly inclined orientations to resist sustained tension loads shall be performed by personnel certified by an applicable certification program in accordance with ACI 318-14 17.8.2.2 or 17.8.2.3; or ACI 318-11 D.9.2.2 or D.9.2.3, as applicable.

5.20 Red Head A7+ Adhesive Anchors may be used to resist tension and shear forces for floor (vertically

down), wall (horizontal) and overhead installations with concrete temperatures between 14°F and 110°F. Horizontal and overhead applications are to be used with the $^{3}/_{8}$-inch (9.5 mm) through $1^{1}/_{4}$-inch (31 mm) diameter threaded rods and reinforcing bars. The adhesive must be injected directly to the back end of the hole using extension tubing (E916-6) for the ½-inch- through $1^{1}/_{4}$-inch-diameter anchors, and extension tubing (E25-6) for the $^{3}/_{8}$-inch-diameter anchors. Alternatively, the $^{5}/_{8}$-inch- (16 mm) through $1^{1}/_{4}$-inch-diameter (31 mm) threaded rod and reinforcing bars may be installed with a Red Head piston plug. See the MPII in Figure 3 of this report for temperature and installation requirements.

5.21 Anchors may be used for applications where the concrete temperature can rise from 40°F (or less) to 80°F (or higher) within a 12-hour period. Such applications may include but are not limited to anchorage of building facade systems and other applications subject to direct sun exposure.

5.22 Red Head A7+ Adhesive is manufactured under a quality-control program with inspections by ICC-ES.

6.0 EVIDENCE SUBMITTED

Data in accordance with the ICC-ES Acceptance Criteria for Post-installed Adhesive Anchors in Concrete Elements (AC308), dated June 2016, which incorporates requirements in ACI 355.4-11.

7.0 IDENTIFICATION

Red Head A7+ Adhesive is identified by labels on the adhesive cartridges bearing the adhesive manufacturer's name (ITW Commercial Construction North America) and address (Glendale Heights, Illinois), the product name (Red Head A7+), best-used-by expiration date, and the evaluation report number (ESR-3903).

FIGURE 1—RED HEAD EPCON A7+ ADHESIVE CARTRIDGES, DISPENSING TOOLS, MIXING NOZZLES, HOLE CLEANING BRUSHES AND HOLE PLUGS

TABLE 1—STEEL DESIGN INFORMATION FOR U.S. CUSTOMARY UNIT THREADED ROD [(1)]

CHARACTERISTIC		SYMBOL	UNITS	NOMINAL ROD DIAMETER (inch)						
				$^3/_8$	$^1/_2$	$^5/_8$	$^3/_4$	$^7/_8$	1	$1^1/_4$
Threaded rod effective cross-sectional area		A_{se}	inch²	0.078	0.142	0.226	0.335	0.462	0.606	0.969
Carbon Steel A36	Nominal steel strength in tension	N_{sa}	lb	4,500	8,230	13,110	19,400	26,780	35,130	56,210
	Nominal steel strength in shear	V_{sa}	lb	2,700	4,940	7,870	11,640	16,070	21,080	33,730
	Strength reduction factor for tension, steel failure mode	ϕ	-	0.75	0.75	0.75	0.75	0.75	0.75	0.75
	Strength reduction factor for shear, steel failure mode[1]	ϕ	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.70	0.70	0.70	0.70	0.70	0.70	0.70
Carbon Steel A193 B7	Nominal steel strength in tension	N_{sa}	lb	9,690	17,740	28,250	41,810	57,710	75,710	121,140
	Nominal steel strength in shear	V_{sa}	lb	5,810	10,640	16,950	25,090	34,630	45,430	72,680
	Strength reduction factor for tension, steel failure mode	ϕ	-	0.75	0.75	0.75	0.75	0.75	0.75	0.75
	Strength reduction factor for shear, steel failure mode[1]	ϕ	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.70	0.70	0.70	0.70	0.70	0.70	0.70
Stainless Steel F593	F593 CW1 nominal steel strength in tension	N_{sa}	lb	7,365	13,480	21,470	-	-	-	-
	F593 CW1 nominal steel strength in shear	V_{sa}	lb	3,680	6,740	10,735	-	-	-	-
	F593 CW2 nominal steel strength in tension	N_{sa}	lb	-	-	-	25,385	35,110	46,055	73,645
	F593 CW2 nominal steel strength in shear	V_{sa}	lb	-	-	-	12,690	17,555	23,030	36,820
	Strength reduction factor for tension, steel failure mode[1]	ϕ	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Strength reduction factor for shear, steel failure mode	ϕ	-	0.60	0.60	0.60	0.60	0.60	0.60	0.60
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.70	0.70	0.70	0.70	0.70	0.70	0.70

For **SI:** 1 inch = 25.4mm, 1 lbf = 4.45N, 1ft-lbf = 1.356 N-M, 1 psi = 0.006895 MPa.

[1]The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 9.2 are used. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4.

TABLE 2—CONCRETE BREAKOUT DESIGN INFORMATION FOR U.S. CUSTOMARY UNIT THREADED ROD [(1)]

CHARACTERISTIC	SYMBOL	UNITS	NOMINAL ROD DIAMETER (inch)						
			$^3/_8$	$^1/_2$	$^5/_8$	$^3/_4$	$^7/_8$	1	$1^1/_4$
Effectiveness factor for uncracked concrete	k_{uncr}	-	24	24	24	24	24	24	24
Effectiveness factor for cracked concrete	k_{cr}	-	17	17	17	17	17	17	17
Minimum concrete thickness	h_{min}	in.	$h_{ef} + 1^1/_4$		$h_{ef} + 2d_o$				
Anchor embedment depth - minimum	$h_{ef,min}$	in.	$2^3/_8$	$2^3/_4$	$3^1/_8$	$3^1/_2$	$3^1/_2$	4	5
Minimum spacing	s_{min}	in.	$^{15}/_{16}$	$1^1/_2$	$2^1/_2$	3	$3^1/_2$	4	5
Minimum edge distance	c_{min}	in.	$^{15}/_{16}$	$1^1/_2$	$2^1/_2$	3	$3^1/_2$	4	5
Critical edge distance	c_{ac}	in.	See Section 4.1.10 of this report						
Strength reduction factor for tension, concrete failure mode[1]	ϕ	Cond. B	0.65	0.65	0.65	0.65	0.65	0.65	0.65
Strength reduction factor for shear, concrete failure mode[1]	ϕ	Cond. B	0.70	0.70	0.70	0.70	0.70	0.70	0.70

For **SI:** 1 inch = 25.4mm, 1 lbf = 4.45N, 1ft-lbf = 1.356 N-M, 1 psi = 0.006895 MPa.

[1]The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 9.2 are used and the requirements of ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, for Condition B are met. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4 for Condition B.

TABLE 3—RED HEAD EPCON A7+ ADHESIVE ANCHOR BOND STRENGTH DESIGN INFORMATION FOR U.S. CUSTOMARY UNIT THREADED ROD [1,4]

CHARACTERISTIC		SYMBOL	UNITS	NOMINAL ROD DIAMETER (inch)						
				3/8	1/2	5/8	3/4	7/8	1	1 1/4
Anchor embedment depth - minimum		h_{ef}	in.	2 3/8	2 3/4	3 1/8	3 1/2	3 1/2	4	5
Anchor embedment depth - maximum		h_{ef}	in.	7 1/2	10	12 1/2	15	17 1/2	20	25
Temperature Range A[2]	Characteristic Bond Strength for Uncracked Concrete	$\tau_{k,uncr}$	psi	1,770	1,770	1,770	1,770	1,490	1,490	1,490
	Characteristic Bond Strength for Cracked Concrete	$\tau_{k,cr}$	psi	1,060	790	860	890	695	655	585
Temperature Range B[3]	Characteristic Bond Strength for Uncracked Concrete	$\tau_{k,uncr}$	psi	1,275	1,275	1,275	1,275	1,080	1,080	1,080
	Characteristic Bond Strength for Cracked Concrete	$\tau_{k,cr}$	psi	765	570	620	640	500	475	420
Continuous Inspection	Strength Reduction Factor - Dry Concrete	$\phi_{dry,ci}$	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Strength Reduction Factor – Water-Saturated Concrete	$\phi_{sat,ci}$	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Strength Reduction Factor - Water-Filled Holes	$\phi_{wf,ci}$	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Strength Reduction Factor - Submerged Concrete	$\phi_{sub,ci}$	-	0.65	0.55	0.55	0.65	0.65	0.55	0.65
Periodic Inspection	Strength Reduction Factor - Dry Concrete	$\phi_{dry,pi}$	-	0.55	0.55	0.55	0.55	0.55	0.55	0.65
	Strength Reduction Factor – Water-Saturated Concrete	$\phi_{sat,pi}$	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Strength Reduction Factor - Water-Filled Holes	$\phi_{wf,pi}$	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Strength Reduction Factor - Submerged Concrete	$\phi_{sub,pi}$	-	0.65	0.45	0.45	0.65	0.55	0.45	0.65
Reduction factor for seismic tension		$\alpha_{N,seis}$	-	0.89	0.75	0.76	0.66	0.77	0.80	0.80

For **SI:** 1 inch = 25.4mm, 1 lbf = 4.45N, 1ft-lbf = 1.356 N-M, 1 psi = 0.006895 MPa.

[1]Bond strength values correspond to concrete compressive strengths ranging from 2,500 psi to 8,000 psi [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1].
[2]Temperature range A: Maximum short term temperature of 130°F and maximum long term temperature of 110°F.
[3]Temperature range B: Maximum short term temperature of 176°F and maximum long term temperature of 110°F.
[4]For structures assigned to IBC or IRC Seismic Design Category C, D, E, or F, bond strength values must be multiplied by $\alpha_{N,seis}$.

TABLE 4—STEEL DESIGN INFORMATION FOR U.S. CUSTOMARY UNIT REINFORCING BARS [1]

CHARACTERISTIC		SYMBOL	UNITS	NOMINAL ROD DIAMETER (inch)							
				No. 3	No. 4	No. 5	No. 6	No. 7	No. 8	No. 9	No. 10
Nominal bar diameter		d	in.	$^3/_8$	$^1/_2$	$^5/_8$	$^3/_4$	$^7/_8$	1	$1^1/_8$	$1^1/_4$
Reinforcing bar effective cross-sectional area		A_{se}	inch²	0.11	0.2	0.31	0.44	0.6	0.79	1.00	1.27
ASTM 615 Grade 60	Nominal steel strength in tension	N_{sa}	lb	9,900	18,000	27,900	39,600	54,000	71,100	90,000	114,300
	Nominal steel strength in shear	V_{sa}	lb	5,940	10,800	16,740	23,760	32,400	42,660	54,000	68,580
	Strength reduction factor for tension, steel failure mode	ϕ	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Strength reduction factor for shear, steel failure mode[1]	ϕ	-	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
	Reduction factor for seismic shear	$\alpha_{V,seis}$	-	0.91	0.91	0.91	0.90	0.90	0.75	0.75	0.75

For **SI:** 1 inch = 25.4mm, 1 lbf = 4.45N, 1ft-lbf = 1.356 N-M, 1 psi = 0.006895 MPa.

[1]The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 9.2 are used. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4.

TABLE 5—CONCRETE BREAKOUT DESIGN INFORMATION FOR U.S. CUSTOMARY UNIT REINFORCING BARS [1,2]

CHARACTERISTIC	SYMBOL	UNITS	NOMINAL ROD DIAMETER (inch)							
			No. 3	No. 4	No. 5	No. 6	No. 7	No. 8	No. 9	No. 10
Effectiveness factor for uncracked concrete	k_{uncr}	-	24	24	24	24	24	24	24	24
Effectiveness factor for cracked concrete	k_{cr}	-	17	17	17	17	17	17	17	17
Minimum concrete thickness	h_{min}	in.	$h_{ef} + 1^1/_4$			$h_{ef} + 2d_o$				
Anchor embedment depth - minimum	$h_{ef,min}$	in.	$2^3/_8$	$2^3/_4$	$3^1/_8$	$3^1/_2$	$3^1/_2$	4	$4^1/_2$	5
Minimum spacing	s_{min}	in.	$^{15}/_{16}$	$1^1/_2$	$2^1/_2$	3	$3^1/_2$	4	$4^1/_2$	5
Minimum edge distance	c_{min}	in.	$^{15}/_{16}$	$1^1/_2$	$2^1/_2$	3	$3^1/_2$	4	$4^1/_2$	5
Critical edge distance	c_{ac}	in.		See Section 4.1.10 of this report						
Strength reduction factor for tension, concrete failure mode[1]	ϕ	Cond. B	0.65	0.65	0.65	0.65	0.65	0.65	0.65	0.65
Strength reduction factor for shear, concrete failure mode[1]	ϕ	Cond. B	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70

For **SI:** 1 inch = 25.4mm, 1 lbf = 4.45N, 1ft-lbf = 1.356 N-M, 1 psi = 0.006895 MPa.

[1]The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 9.2 are used and the requirements of ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, for Condition B are met. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4 for Condition B.

[2]The value of f'_c used for calculation must be limited to maximum 8,000 psi (55 MPa) in accordance with ACI 318-14 17.2.7 or ACI 318-11 D.3.7, as applicable.

TABLE 6—RED HEAD EPCON A7+ ADHESIVE ANCHOR BOND STRENGTH DESIGN INFORMATION FOR U.S. CUSTOMARY UNIT REINFORCING STEEL [1,4]

CHARACTERISTIC		SYMBOL	UNITS	NOMINAL ROD DIAMETER (inch)							
				No. 3	No. 4	No. 5	No. 6	No. 7	No. 8	No. 9	No. 10
Anchor embedment depth - minimum		h_{ef}	in.	$2^3/_8$	$2^3/_4$	$3^1/_8$	$3^1/_2$	$3^1/_2$	4	$4^1/_2$	5
Anchor embedment depth - maximum		h_{ef}	in.	$7^1/_2$	10	$12^1/_2$	15	$17^1/_2$	20	$22^1/_2$	25
Temperature Range A[2]	Characteristic Bond Strength for Uncracked Concrete	$\tau_{k,uncr}$	psi	1,675	1,935	1,900	1,700	1,635	1,615	1,585	1,550
	Characteristic Bond Strength for Cracked Concrete	$\tau_{k,cr}$	psi	755	755	755	585	585	585	585	585
Temperature Range B[3]	Characteristic Bond Strength for Uncracked Concrete	$\tau_{k,uncr}$	psi	1,210	1,400	1,370	1,230	1,180	1,165	1,145	1,120
	Characteristic Bond Strength for Cracked Concrete	$\tau_{k,cr}$	psi	545	545	545	420	420	420	420	435
Continuous Inspection	Strength Reduction Factor - Dry Concrete	$\phi_{dry,ci}$	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Strength Reduction Factor – Water-Saturated Concrete	$\phi_{sat,ci}$	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Strength Reduction Factor - Water-Filled Holes	$\phi_{wf,ci}$	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Strength Reduction Factor - Submerged Concrete	$\phi_{sub,aci}$	-	0.65	0.55	0.55	0.65	0.65	0.55	0.55	0.65
Periodic Inspection	Strength Reduction Factor - Dry Concrete	$\phi_{dry,pi}$	-	0.55	0.55	0.55	0.55	0.55	0.55	0.55	0.65
	Strength Reduction Factor – Water-Saturated Concrete	$\phi_{sat,pi}$	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Strength Reduction Factor - Water-Filled Holes	$\phi_{wf,pi}$	-	0.65	0.65	0.65	0.65	0.65	0.65	0.65	0.65
	Strength Reduction Factor - Submerged Concrete	$\phi_{sub,pi}$	-	0.65	0.45	0.45	0.65	0.55	0.45	0.45	0.65
Reduction factor for seismic tension		$\alpha_{N,seis}$	-	0.92	0.92	0.92	0.82	0.82	0.82	0.82	0.82

For **SI:** 1 inch = 25.4mm, 1 lbf = 4.45N, 1ft-lbf = 1.356 N-M, 1 psi = 0.006895 MPa.

[1]Bond strength values correspond to concrete compressive strengths ranging from 2,500 psi to 8,000 psi [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1].

[2]Temperature range A: Maximum short term temperature of 130°F and maximum long term temperature of 110°F.

[3]Temperature range B: Maximum short term temperature of 176°F and maximum long term temperature of 110°F.

[4]For structures assigned to IBC or IRC Seismic Design Category C, D, E, or F, bond strength values must be multiplied by $\alpha_{N,seis}$.

TABLE 7—EXAMPLE RED HEAD EPCON A7+ ADHESIVE ALLOWABLE STRESS DESIGN VALUES (ASD) FOR ILLUSTRATIVE PURPOSES

Anchor Diameter (d)	Min/Max Embedment Depth, h_{ef} (in)	Char. Bond Strength $\tau_{k,uncr}$ (psi)	Allowable Tension Load (lb) 2500psi- 8000psi	Controlling Failure Mode
3/8	2 3/8	1,770	1,929	Concrete
	7 1/2		2,280	Steel
1/2	2 3/4	1,770	2,403	Concrete
	10		4,171	Steel
5/8	3 1/8	1,770	2,911	Concrete
	12 1/2		6,644	Steel
3/4	3 1/2	1,770	3,451	Concrete
	15		9,831	Steel
7/8	3 1/2	1,490	3,451	Concrete
	17 1/2		13,571	Steel
1	4	1,490	4,216	Concrete
	20		17,802	Steel
1 1/4	5	1,490	5892	Concrete
	25		28,485	Steel

For **SI:** 1 inch = 25.4mm, 1 lbf = 4.45N, 1ft-lbf = 1.356 N-M, 1 psi = 0.006895 MPa.

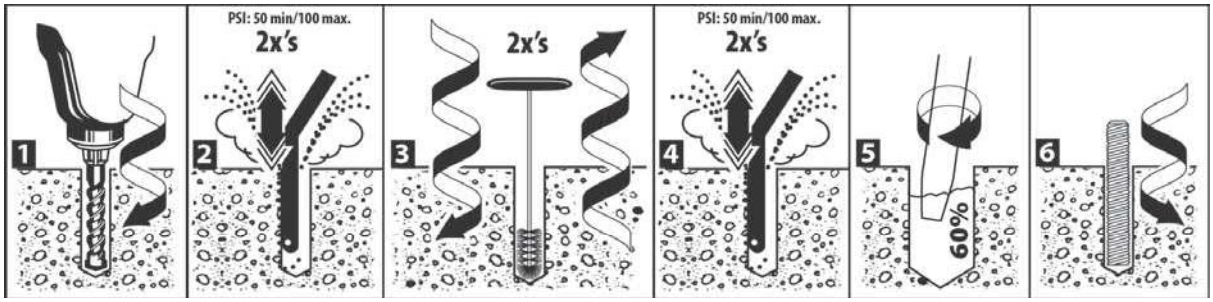
This table was developed based on the following conditions:
[1]Single anchor with static tension only, A36 threaded rod
[2]Vertical downward installation direction
[3]Inspection regimen = Periodic
[4]Installation temperature = 30°F to 90°F
[5]Long term temperature = 110°F
[6]Short term temperature = 130°F
[7]Dry hole condition (carbide drilled hole)
[8]Embedment = hef (min/max for each diameter)
[9]Concrete determined to remain uncracked for the life of the anchorage
[10]Load combinations from ACI 318-11 Section 9.2 (no seismic loading)
[11]30% dead load and 70% live load, controlling load combination 1.2D + 1.6L
[12]Calculation of weighted average for α = 0.3*1.2 + 0.7*1.6 = 1.48
[13]f'_c = 2,500 psi (normal weight concrete)
[14]$c_{a1} = c_{a2} \geq c_{ac}$
[15]$h \geq h_{min}$

Illustrative Procedure to Calculate Allowable Stress Design Tension Value:
Red Head Epcon A7+ Adhesive Anchor $^1/_2$-inch diameter, using an embedment of $4^1/_2$ inches, assuming the conditions given in Table 7 (for use with the 2012 IBC, based on ACI 318-11 Appendix D). Applied tension load, N_{ua} = 4,000 lbs.

	PROCEDURE	***CALCULATION***
Step 1	Calculate steel strength of a single anchor in tension per ACI 318-11 D.5.1.2 and Table 1 of this report.	ϕN_{sa} = 0.75*8,230 = 6,173 lbs steel strength
Step 2	Calculate concrete breakout strength of a single anchor in tension per ACI 318-11 D.5.2 and Table 2 of this report.	$N_b = k_{c,uncr} * \lambda_a \sqrt{f'_c}\ h_{ef}^{1.5} = 24 * \sqrt{2,500} * 4.5^{1.5}$ N_b = 11,455 lbs $\phi N_{cb} = \phi\ A_{NC}/A_{NC0}\ \psi_{ed,N}\ \psi_{c,N}\ \psi_{cp,N}\ N_b$ ϕN_{cb} = 0.65*1.0*1.0*1.0*1.0*11,455 ϕN_{cb} = 7,446 lbs concrete breakout strength
Step 3	Calculate bond strength of a single anchor in tension per ACI 318-11 D.5.5 and Table 3 of this report.	$N_{ba} = {}^*\lambda_a\ \tau_{k,uncr}\ \pi d h_{ef}$ N_{ba} = 1.0*1,770*3.14*0.5*4.5 N_{ba} = 12,505 lbs $\phi N_a = \phi\ A_{Na}/A_{Na0}\ \psi_{ed,Na}\ \psi_{cp,Na}\ N_{ao}$ ϕN_a = 0.65*1.0*1.0*1.0*12,505 ϕN_a = 8,128 lbs bond strength
Step 4	Determine compliance with required anchor strength per ACI 318-11 D.4.1.	ϕN_{sa} = 6,173 lbs > N_{ua} = 4,000 lbs ϕN_{cb} = 7,446 lbs > N_{ua} = 4,000 lbs ϕN_a = 8,128 lbs > N_{ua} = 4,000 lbs
Step 5	Calculate allowable stress design conversion factor for loading condition per ACI 318-11 Section 9.2.	$\alpha = 1.2D + 1.6L = 1.2(0.3) + 1.6(0.7) = 1.48$
Step 6	Calculate allowable stress design value per Section 4.2 of this report.	$T_{allowable,ASD} = \phi N_n / \alpha$ = 6,173 lbs/1.48 $T_{allowable,ASD}$ = 4,171 lbs allowable stress design

FIGURE 2—EXAMPLE DESIGN CALCULATION

RED HEAD EPCON A7+ ADHESIVE ANCHOR INSTALLATION INSTRUCTIONS



* Water-saturated concrete, water-filled holes and submerged concrete applications require 4x's air, 4x's brushing and 4x's air

1) ▪ Use a rotary hammer drill or pneumatic air drill with a carbide drill bit complying to ANSI B212.15-1994 tolerance requirements. Drill hole to the required embedment depth. See attached table for drill bit specifications and min/maximum embedment depths.
▪ Installations may be used with maximum 1-1/4" diameter rods/rebar for floor, wall and overhead applications.
▪ Per construction specification, adhere to minimum spacing, minimum edge distance, and minimum member thickness.

2) ▪ For dry holes, oscillate a clean air nozzle in and out of the dry hole two times, for a total of two seconds, starting at the bottom of the hole with contaminant-free compressed air, exhausting hole until visually clean (i.e., no dust, debris, etc.)
▪ For water-saturated concrete and water-filled hole applications, oscillate a clean air nozzle in and out of the damp, water-filled or submerged hole four times, for a total of four seconds, starting at the bottom of the hole with contaminant-free compressed air, exhausting hole until visually clean (i.e., no dust, debris, etc.)
▪ If required, use an extension on the end of the air nozzle to reach the bottom of the hole.

3) ▪ Select an appropriately sized Red Head brush for the anchor diameter. Brush must be checked for wear before use. See attached table for brush specifications, including minimum diameter.
▪ Insert the brush into the hole with a clockwise motion. For every ½" forward advancement, complete one full turn until bottom of hole is reached. For faster and more suitable cleaning, attach the brush to a drill.
▪ Using a clockwise motion, for every full turn of the brush, pull the brush ½" out of the hole.
▪ For dry holes, twist/spin the brush two times in/out of the hole.
▪ For water-saturated concrete and water-filled hole applications, twist/spin the brush four times in/out of the hole.
▪ If required, use a wire brush extension (part nos. ESDS-38 or EHAN-38) to reach the bottom of the hole.
▪ Air clean the dust off the brush to prevent clogging of the brush.

4) ▪ For dry holes, oscillate a clean air nozzle in and out of the dry hole two times, for a total of two seconds, starting at the bottom of the hole with contaminant-free compressed air, exhausting hole until visually clean (i.e., no dust, debris, etc.)
▪ For water-saturated concrete and water-filled hole applications, oscillate a clean air nozzle in and out of the damp, water-filled or submerged hole four times, for a total of four seconds, starting at the bottom of the hole with contaminant-free compressed air, exhausting hole until visually clean (i.e., no dust, debris, etc.)

5) ▪ Review the Safety Data Sheet (SDS) before use.
▪ Check the "Use By" date on the cartridge and that the cartridge has been stored in out of direct sunlight.
▪ Review the gel time/cure time chart, based on the temperature at time of installation, in order to determine tool, cartridge and nozzle requirements.
▪ Assemble the Red Head supplied cartridge and nozzle. Do not modify or remove mixing elements in nozzle.
▪ If nozzle does not reach the bottom of the hole, use Red Head E25-6 extension tubing (0.44" O.D.) positioned on the end of nozzle or use the S75EXT (nozzle extension) on the end of the S75 nozzle.
▪Place the assembly into a hand injection tool or a pneumatic injection tool.
▪ Dispense mixed adhesive outside of hole until uniform color is achieved.
▪ During installations, concrete must be between 14 and 110 degrees F, or artificially maintained.
▪ Insert the nozzle to the bottom of the hole and inject the adhesive at an angle, leaving the nozzle tip always slightly below the fill level.
▪ In a slow circular direction, work the adhesive into the sides of the hole, filling slowly to ensure proper adhesive distribution, until the hole is approximately 60% filled.
▪ For holes that contain water, keep injecting the adhesive below the water in order to displace the water upward.

HORIZONTAL AND OVERHEAD INJECTION OF ADHESIVE:
▪ For 3/8" thru 1¼" diameter anchors installed horizontal and overhead, the adhesive may be injected directly to the end of the hole using the Red Head E25-6 extension tubing (0.44" O.D.) for the $^{3}/_{8}$" diameter anchors, and Red Head E916-6 extension tubing (0.56" O.D.) for the ½" thru 1¼" diameter anchors.

▪ Alternatively, for 5/8" diameter rod (#5 rebar) and larger anchors installed horizontal and overhead, assemble Red Head E916-6 extension tubing and appropriate sized piston plug on end of tubing:
PL-5834 for $^{5}/_{8}$" & ¾" diameter rod (No. 5 and No. 6 rebar)
PL-7810 for $^{7}/_{8}$" & 1" diameter rod (No. 7 and No. 8 rebar)
PL-1250 for 1-$^{1}/_{4}$" diameter rod (No. 9 and No. 10 rebar)

▪ The use of the Red Head pneumatic tool may be required for larger diameter anchors and/or deeper embedment installations at temperatures up to 110 degrees F.

6) ▪ Immediately insert the oil, rust and scale free rod/rebar assembly to the required embedment depth, using a counterclockwise motion to ensure proper adhesive distribution.
▪ The anchor rod/rebar must be marked with the required embedment depth.
▪ For wall (horizontal) and overhead installations with concrete or adhesive over 70 degrees F, the anchor rod/rebar must be marked with the required embedment depth and assembled with a Red Head hole plug positioned on the rod/rebar at the required embedment depth.
▪ After installing the anchor, the gap between the rod and the concrete must be completely filled with adhesive. The adhesive must fill voids, crevices and uniformly coat the rod and concrete.
▪ After installation, do not disturb the anchor until the full cure time has elapsed. Overhead installations must be supported until full cure time has elapsed.
▪ Adhesive must be fully cured before applying any load or torque. Do not over torque the anchor as this could adversely affect its performance.

FIGURE 3—RED HEAD EPCON A7+ ADHESIVE INSTALLATION INSTRUCTIONS

SPECIFICATIONS FOR INSTALLATION OF RED HEAD EPCON S7 ADHESIVE ANCHORS IN CONCRETE

FOR INSTALLATION USING U.S. CUSTOMARY UNIT THREADED ROD

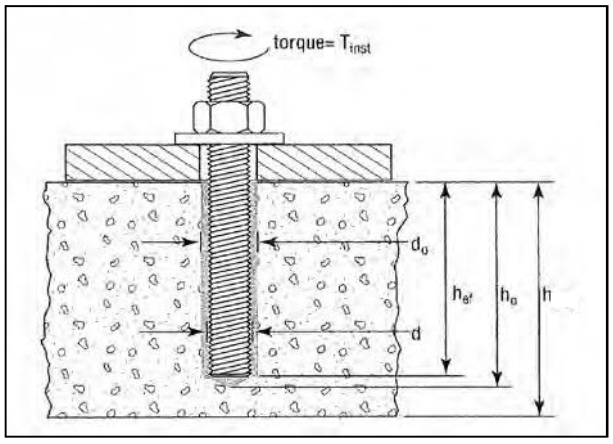
CHARACTERISTIC	SYMBOL	UNITS	NOMINAL ROD DIAMETER (inch)						
			3/8	1/2	5/8	3/4	7/8	1	1 1/4
Nominal carbide bit diameter	-	in.	7/16	9/16	3/4	7/8	1	1 1/8	1 3/8
Anchor embedment depth - minimum	$h_{ef,\ min}$	in.	2 3/8	2 3/4	3 1/8	3 1/2	3 1/2	4	5
Anchor embedment depth - maximum	$h_{ef,\ max}$	in.	7 1/2	10	12 1/2	15	17 1/2	20	25
Minimum spacing	s_{min}	in.	15/16	1 1/2	2 1/2	3	3 1/2	4	5
Minimum edge distance	c_{min}	in.	15/16	1 1/2	2 1/2	3	3 1/2	4	5
Minimum concrete thickness	h_{min}	in.	h_{ef} + 1 1/4		h_{ef} + $2d_o$				
Maximum tightening torque for pretension clamping	T_{inst}	ft lb	9	16	47	70	90	110	370

FOR INSTALLATION USING U.S. CUSTOMARY UNIT REINFORCING BARS

CHARACTERISTIC	SYMBOL	UNITS	NOMINAL REBAR DIAMETER (inch)							
			No. 3	No. 4	No. 5	No. 6	No. 7	No. 8	No. 9	No. 10
Nominal carbide bit diameter	-	in.	7/16	5/8	3/4	7/8	1	1 1/8	1 1/4	1 3/8
Anchor embedment depth - minimum	$h_{ef,\ min}$	in.	2 3/8	2 3/4	3 1/8	3 1/2	3 1/2	4	4 1/2	5
Anchor embedment depth - maximum	$h_{ef,\ max}$	in.	7 1/2	10	12 1/2	15	17 1/2	20	22 1/2	25
Minimum spacing	s_{min}	in.	15/16	1 1/2	2 1/2	3	3 1/2	4	4 1/2	5
Minimum edge distance	c_{min}	in.	15/16	1 1/2	2 1/2	3	3 1/2	4	4 1/2	5
Minimum concrete thickness	h_{min}	in.	h_{ef} + 1 1/4			h_{ef} + $2d_o$				

For **SI:** 1 inch = 25.4mm, 1 lbf = 4.45N, 1ft-lbf = 1.356N-m, 1psi = 0.006895MPa.

ANCHOR INSTALLATION



BRUSH SPECIFICATIONS

Anchor diameter (in)	Brush Part No.	Minimum brush diameter (in)
3/8 No. 3	SB038	0.563
1/2 No. 4	SB012	0.675
5/8 No. 5	SB058	0.900
3/4 No. 6	SB034	1.125
7/8 No. 7	SB078	1.350
1 No. 8, and No. 9	SB010	1.463
1 1/4 No. 10	SB125	1.575

FIGURE 3—ITW RED HEAD S7 ADHESIVE INSTALLATION INSTRUCTIONS (Continued)

MIXING NOZZLE, EXTENSION TUBING & PISTON PLUG SPECIFICATIONS FOR HORIZONTAL AND OVERHEAD INSTALLATION ACCESSORIES

Anchor diameter (in)	Mixing nozzle	Extension nozzle	Extension tubing	Piston plug
3/8	A24S	N/A[1]	E25-6[4]	N/A[1]
	S55			
1/2	A24S	N/A[1]	E916-6[4]	N/A[1]
	S55			
5/8	A24S	N/A[1]	E916-6[4]	N/A[1]
	S55			
	S75	S75EXT	E916-6[2]	PL-5834[3]
3/4	S55	N/A[1]	E916-6	N/A[1]
	S75	S75EXT	E916-6[2]	PL-5834[2]
7/8	S55	N/A[1]	E916-6	N/A[1]
	S75	S75EXT	E916-6[2]	PL-7810[3]
1	S55	N/A[1]	E916-6	N/A[1]
	S75	S75EXT	E916-6[2]	PL-7810[3]
1 1/4	S55	N/A[1]	E916-6	N/A[1]
	S75	S75EXT	E916-6[2]	PL-1250[3]

[1]N/A = not available
[2]For use with the mixing nozzle S75 and extension nozzle S75EXT for horizontal and overhead installation in accordance with Section 4.3 of this report.
[3]For use with the mixing nozzle S75, extension nozzle S75EXT and extension tubing E916-6 for horizontal and overhead installation in accordance with Section 4.3 of this report.
[4]For use with the mixing nozzle A24S or S55 for horizontal and overhead installation in accordance with Section 4.3 of this report.

CURE TIMES AND GEL TIMES FOR RED HEAD EPCON A7+ ADHESIVE

Concrete Temperature (°F)[1,2]	Gel Time[3]	Cure Time[4]
110	1.5 minutes	45 minutes
90	5 minutes	45 minutes
70	10 minutes	45 minutes
50	16 minutes	90 minutes
32	35 minutes	4 hours
14	35 minutes	24 hours

For **SI:** t° (°F-32) X .555 = °C.

[1]Adhesive must be installed in concrete temperatures within the noted range or artificially maintained at the noted temperature.
[2]For concrete temperatures between 14°F and 32°F, adhesive must be maintained at a minimum of 32°F during installation.
[3]Gel time is the maximum time from the end of mixing to when the insertion of the anchor into the adhesive shall be completed and is based upon the adhesive and concrete temperatures noted.
[4]Cure time is the minimum time from the end of gel time to when the anchor maybe torque or loaded. Anchors are to be undisturbed during the cure time.

FIGURE 3—RED HEAD A7+ ADHESIVE INSTALLATION INSTRUCTIONS (Continued)

ICC-ES Evaluation Report

ESR-3903 FBC Supplement

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Section: 03 16 00—Concrete Anchors

DIVISION: 05 00 00—METALS
Section: 05 05 19—Post-Installed Concrete Anchors

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EVALUATION SUBJECT:

ITW RED HEAD A7+ ADHESIVE ANCHORING SYSTEM FOR CRACKED AND UNCRACKED CONCRETE

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that Red Head A7+ Adhesive Anchoring System for Cracked and Uncracked Concrete, recognized in ICC-ES master evaluation report ESR-3903, has also been evaluated for compliance with the codes noted below.

Compliance with the following codes:

- 2014 *Florida Building Code—Building*
- 2014 *Florida Building Code—Residential*

2.0 PURPOSE OF THIS SUPPLEMENT

This supplement is issued to indicate that the Red Head A7+ Adhesive Anchoring System for Cracked and Uncracked Concrete described in Sections 2.0 through 7.0 of the master report, ESR-3903, complies with the 2014 *Florida Building Code—Building* and the 2014 *Florida Building Code—Residential*, when designed and installed in accordance with the 2012 *International Building Code®* (IBC) provisions noted in the master evaluation report under the following conditions:

- Design wind loads must be based on Section 1609 of the 2014 *Florida Building Code—Building* or Section 301.2.1.1 of the 2014 *Florida Building Code—Residential*, as applicable.
- Load combinations must be in accordance with Section 1605.2 or Section 1605.3 of the 2014 *Florida Building Code—Building*, as applicable.

Use of the Red Head A7+ Adhesive Anchoring System with stainless steel threaded rod materials has also been found to be in compliance with the High-Velocity Hurricane Zone provisions of the 2014 *Florida Building Code—Building* and the 2014 *Florida Building Code—Residential* when the following condition is met:

The design wind loads for use of the anchors in a High-Velocity Hurricane Zone are based on Section 1620 of the *Florida Building Code—Building.*

Use of the the Red Head A7+ Adhesive Anchoring System with carbon steel threaded rod materials and reinforcing bars for compliance with the High-velocity Hurricane Zone provisions of the 2014 *Florida Building Code—Building* and the 2014 *Florida Building Code—Residential* has not been evaluated and is outside the scope of this supplemental report.

For products falling under Florida Rule 9N-3, verification that the report holder's quality-assurance program is audited by a quality-assurance entity approved by the Florida Building Commission for the type of inspections being conducted is the responsibility of an approved validation entity (or the code official, when the report holder does not possess an approval by the Commission).

This supplement expires concurrently with the master report, issued December 2016 and corrected December 5, 2016.

DAYTON® SUPERIOR®

No Waterstop in applications for this job

A46 LOOP TIE

Light Forming Tie

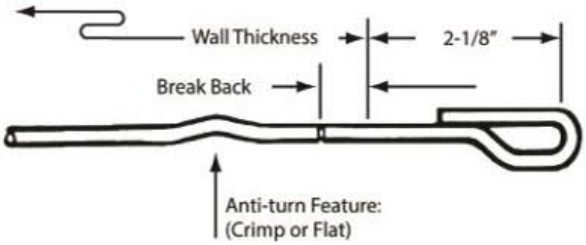
TECHNICAL DATA SHEET

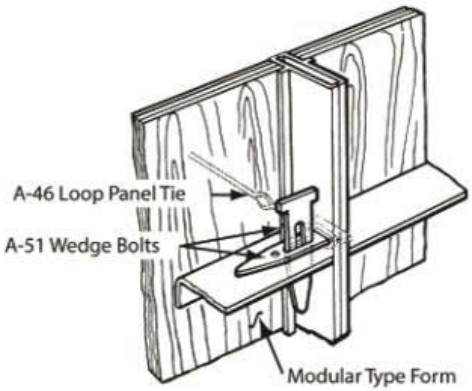
DESCRIPTION

A46 Loop Ties are manufactured from high strength steel.

APPLICATION

A46 Loop Ties are designed for securing and spacing modular type forms.





FEATURES

- Standard units are fabricated with 2-1/8" ends and 1" breakback.
- Other breakbacks are available on special order.
- All loop ties can be manufactured with plastic cones and water-resistant washers.
- All loop ties can be manufactured with a tight fitting neoprene washer collated near the center of the tie. The water-resistant washer is designed to help eliminate water seepage along the tie by breaking the surface continuity of the wire.
- Safe Working Load is 2,250 lbs. for standard or 3,000 lbs for heavy ties which provides a factor of safety of approximately 2 to 1.
- A46 Stainless Steel Loop Ties are available for applications that require ties resistant to rusting or other similar corrosion.
- Due to the ductility of the type 304 stainless steel wire, Dayton Superior cannot guarantee that A46 stainless steel ties will consistently provide proper breakback.

INSTALLATION

1. When erecting modular type forms, the A46 ties are placed in the slots between the form panels.
2. An A51 Wedge Bolt is placed through the adjoining form and into the loop of each tie.
3. A second wedge bolt is then placed through the slot of the first wedge bolt to secure the tie and form together.

RELATED PRODUCTS

- A2 Plastic Cone
- A51 Wedge Bolts
- Steel-Ply® Forms

ORDERING INFORMATION

2,250 LBS SWL STANDARD

Product Code	Description	Weight
118180	6"	0.165 LB
118190	8"	0.185 LB
118210	10"	0.207 LB
118200	12"	0.230 LB
118230	14"	0.250 LB
118240	16"	0.280 LB
118250	18"	0.304 LB
118255	20"	0.340 LB
118220	24"	0.362 LB

TECHNICAL DATA SHEET

3,000 LBS SWL HEAVY DUTY

Product Code	Description	Weight
117070	4"	0.169 LB
118260	6"	0.193 LB
118270	8"	0.208 LB
118275	9"	0.221 LB
118280	10"	0.246 LB
11766	11"	0.270 LB
118290	12"	0.282 LB
100930	13"	0.288 LB
118300	14"	0.290 LB
118305	15"	0.303 LB
118310	16"	0.320 LB
118315	17"	0.330 LB
118320	18"	0.336 LB
143480	19"	0.350 LB
118330	20"	0.364 LB
100940	21"	0.390 LB
118335	22"	0.400 LB
118340	24"	0.420 LB
115210	26"	0.464 LB
143481	27"	0.470 LB
100950	28"	0.475 LB
118350	30"	0.493 LB
100970	32"	0.521 LB
143482	34"	0.560 LB
118360	36"	0.580 LB
143483	38"	0.610 LB
143484	40"	0.640 LB
143485	42"	0.680 LB
143486	44"	0.690 LB
100990	48"	0.710 LB
143487	54"	0.820 LB
125402	60"	0.930 LB
143488	72"	1.150 LB
143489	96"	1.590 LB
143490	120"	2.030 LB

WARRANTY (ACCESSORIES)

Limited Warranty. Dayton warrants, for a period of 60 days from the date of shipment (three years from the date of shipment in the case of formwork, excluding any consumable Products included with such formwork), that Products and any associated application drawings and engineering services provided by Dayton ("Ancillary Services") will be free from defects in material and workmanship and, in the case of custom designed formwork, that the formwork will meet the specifications set forth in the design drawings approved by Dayton and Customer. Any claim under this warranty must be made in writing within such warranty period. If any Product and/or Ancillary Service covered by a timely claim are found to be defective, Dayton will, within a reasonable time, make any necessary repairs or corrections or, at Dayton's option, replace the Product. Unless pre-authorized by Dayton in writing, Dayton will not accept any charges for correcting defects or accept the return of any Product. This warranty will not apply to any Products that have been subjected to misuse, neglect, storage damage, misapplication, accident or any other damage caused by any person other than Dayton, or that have not been maintained in accordance with Dayton's specifications. THIS LIMITED WARRANTY IS IN LIEU OF ALL OTHER WARRANTIES AS TO THE PRODUCTS AND ANCILLARY SERVICES. DAYTON MAKES NO OTHER WARRANTIES OR GUARANTEES, EXPRESS OR IMPLIED, INCLUDING ANY WARRANTY OF MERCHANTABILITY, FITNESS FOR A PARTICULAR PURPOSE OR OTHERWISE. THE REMEDIES SET FORTH IN THIS SECTION ARE CUSTOMER'S EXCLUSIVE REMEDY FOR BREACH OF WARRANTY.

MANUFACTURER

Dayton Superior Corporation
1125 Byers Road
Miamisburg, OH 45342
Customer Service: 888-977-9600
Technical Services: 877-266-7732
Website: www.daytonsuperior.com

File Date: 9/4/2013 1:53:34 PM

TECHNICAL DATA SHEET

DESCRIPTION
Clean Strip J1A is a V.O.C. compliant, ready-to-use, petroleum based, chemically reactive concrete form release. Clean Strip J1A effectively prevents bonding of concrete to forms and formliners. Use of Clean Strip J1A regularly will increase the life of all forms.

USE
Clean Strip J1A chemically releases hardened concrete from steel, aluminum, plywood, and composition forms.

FEATURES
- Chemically reactive
- Positive release
- Increases the life of both wood and steel forms
- Helps prevent deterioration and corrosion of steel
- Light citrus scent

VOC
Less than 250 g/L. Compliant with most all Canadian and U.S. Federal EPA ,OTC, CARB and LADCO regulation for Concrete Form Release agents

Estimating Guide

	Sq. Feet per gallon	Sq. Meters per liter
Steel	2000	49.2
Aluminum	2000	49.2
Medium Density Plywood	1000-1500	24.6-36.9
High Density plywood	2000	49.2
Rough Sawn Lumber 1st coat	700	17.2
2nd coat	1000	24.6
Texture and absorption of forming material will dictate final coverage rate. Prior to coating plywood forms, apply one or two heavy brush coats to edges to ensure ease of form removal.		

Packaging

PRODUCT CODE	PACKAGE	SIZE	
		Gallons	Liters
69201	Pail	5	18.93
69200	Drum	55	208.20
69198	Tote	275	1040.99

STORAGE
Store in dry environment. Shelf life is two years from date of manufacture.

APPLICATION

Surface Preparation:
Surfaces to be treated should be clean and free of water, dust and dirt or residues that might transfer to the final concrete surface.

Placement:
Apply during dry weather with low pressure spray, roller or brush. Apply uniformly in a thin but continuous film to assure proper coverage. For best results apply to forms before each use. Avoid applying too heavily. Wipe excessive material from forms prior to installing forms and concrete placement. Prior to coating plywood forms, apply one or two heavy brush coats to edges to ensure complete water-proofing protection.

CLEAN UP
Sprayers and other equipment may be cleaned with Citrus Cleaner J48 or other solvent degreaser.

LIMITATIONS

FOR PROFESSIONAL USE ONLY
Not for use as a bond breaker or concrete release agent for tiltup concrete construction.
Requires recoating after each use.
Do not apply onto reinforcing steel.
DO NOT OVER APPLY. Over-application could cause excessive surface dusting and/or transfer to concrete.
When any material is to be applied over the newly formed concrete, follow the application instructions of the material manufacturer.

PRECAUTIONS

READ SDS PRIOR TO USING PRODUCT
- Keep material and containers away from high heat, open flames, sparks or other sources of ignition
- Use with adequate ventilation
- Wear protective clothing, gloves and eye protection (goggles, safety glasses and/or face shield)
- Keep out of the reach of children
- Do not take internally
- In case of ingestion, seek medical help immediately
- May cause skin irritation upon contact, especially prolonged or repeated. If skin contact occurs, wash immediately with soap and water and seek medical help as needed.
- If eye contact occurs, flush immediately with clean water and seek medical help as needed
- Dispose of waste material in accordance with federal, state and local requirements

TECHNICAL DATA SHEET

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Technical Services: 877-266-7732
Website: www.daytonsuperior.com

WARRANTY

Dayton Superior Corporation ("Dayton") warrants for 12 months from the date of manufacture or for the duration of the published product shelf life, whichever is less, that at the time of shipment by Dayton, the product is free of manufacturing defects and conforms to Dayton's product properties in force on the date of acceptance by Dayton of the order. Dayton shall only be liable under this warranty if the product has been applied, used, and stored in accordance with Dayton's instructions, especially surface preparation and installation, in force on the date of acceptance by Dayton of the order. The purchaser must examine the product when received and promptly notify Dayton in writing of any non-conformity before the product is used and no later than 30 days after such non-conformity is first discovered. If Dayton, in its sole discretion, determines that the product breached the above warranty, it will, in its sole discretion, replace the non-conforming product, refund the purchase price or issue a credit in the amount of the purchase price. This is the sole and exclusive remedy for breach of this warranty. Only a Dayton officer is authorized to modify this warranty. The information in this data sheet supersedes all other sales information received by the customer during the sales process. THE FOREGOING WARRANTY SHALL BE EXCLUSIVE AND IN LIEU OF ANY OTHER WARRANTIES, EXPRESS OR IMPLIED, INCLUDING WARRANTIES OF MERCHANTABILITY AND FITNESS FOR A PARTICULAR PURPOSE, AND ALL OTHER WARRANTIES OTHERWISE ARISING BY OPERATION OF LAW, COURSE OF DEALING, CUSTOM, TRADE OR OTHERWISE.

Dayton shall not be liable in contract or in tort (including, without limitation, negligence, strict liability or otherwise) for loss of sales, revenues or profits; cost of capital or funds; business interruption or cost of downtime, loss of use, damage to or loss of use of other property (real or personal); failure to realize expected savings; frustration of economic or business expectations; claims by third parties (other than for bodily injury), or economic losses of any kind; or for any special, incidental, indirect, consequential, punitive or exemplary damages arising in any way out of the performance of, or failure to perform, its obligations under any contract for sale of product, even if Dayton could foresee or has been advised of the possibility of such damages. The Parties expressly agree that these limitations on damages are allocations of risk constituting, in part, the consideration for this contract, and also that such limitations shall survive the determination of any court of competent jurisdiction that any remedy provided in these terms or available at law fails of its essential purpose.